# Maes Emlyn, Rhyl

# **Drainage Strategy**

September 2023



Project Information	n
Project:	Maes Emlyn, Rhyl
Report Title:	Drainage Strategy
Client: TACP Architects Ltd	
Instruction:	The instruction to undertake this Drainage Strategy was received from Matthew Gregory of TACP Architects Ltd.
File Ref:	14973-Drainage Strategy-03

Approval Record	
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Document History		
Revision	Date	Comment
01	28/02/2023	First issue
02	21/04/2023	Report update to include ground investigations
03	18/09/2023	Report updated following DCWW correspondence

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### Introduction

Waterco has been instructed to prepare a Drainage Strategy in respect of a proposed residential development at Maes Emlyn, Rhyl, LL18 3SF.

The aim of the Sustainable Drainage Strategy is to identify water management measures, including Sustainable Drainage Systems (SuDS), to provide surface water runoff reduction and treatment. This report has been prepared in accordance with the Welsh Government 'Statutory standards for sustainable drainage systems – designing, constructing, operating and maintaining surface water drainage systems' (2018) – herein referred to as 'the Statutory Standards for SuDS'.

This report has been prepared in consultation with Denbighshire County Council as the Sustainable Drainage Approval Body (SAB) and Dŵr Cymru Welsh Water (DCWW).

# **Existing Conditions**

The site covers an area of approximately 8,900m<sup>2</sup> and is located at National Grid Reference (NGR) 301448, 381587. A location plan and an aerial image are included in Appendix A.

Online mapping (including Google Maps / Google Streetview imagery, accessed February 2023) shows that the site comprises existing residential properties (59 units) with associated access and parking. The site is bordered by residential properties to the north, north-east and west, and a railway line to the east and south. Access to the site is provided from Churton Road to the north.

### **Local Topography**

Topographic levels to metres Above Ordnance Datum (m AOD) have been derived from a 1m resolution Natural Resources Wales (NRW) composite 'Light Detecting and Ranging' (LiDAR) Digital Terrain Model (DTM). A review of LiDAR data shows that the site slopes from approximately 7.08m AOD in the south-west to approximately 6.45m AOD in the east. A LiDAR extract is included as Appendix B.

#### **Ground Conditions**

#### **Published Geology**

The British Geological Survey (BGS) online mapping (1:50,000 scale) indicates that the site is underlain by superficial deposits of blown sand. The superficial deposits are identified as being underlain by the Kinnerton Sandstone Formation.

The geological mapping is available at a scale of 1:50,000 and as such may not be accurate on a site-specific basis.

The closest historical BGS borehole record (BGS Reference: SJ08SW10) is located 320m north-west of the site and is included in Appendix C. The borehole record generally identifies sand to approximately 2.5 metres below ground level (m.bgl) underlain by silty clay to approximately 3.3 m.bgl.



#### **Ground Investigation**

GroundSolve Ltd have undertaken an intrusive ground investigation in January 2023 (project no. GSL2841). The intrusive ground investigation comprised 6No. windowless sample (WS) boreholes advanced to a maximum depth of 5.45 metres below ground level (m.bgl). Windowless sample logs are included as Appendix D.

The ground conditions identified in the intrusive investigation are summarised below:

- Made Ground was observed in WS04 from ground level to 0.50m.bgl.
- Topsoil was observed in all windowless sample boreholes, except WS04, from ground level to a maximum depth of 0.40m.bgl. Topsoil generally consisted of loose dark brown, slightly gravelly silt with rootlets.
- Made Ground was observed in WS03 from a minimum depth of 0.15m.bgl to a maximum depth of 0.70m.bgl. Made Ground generally consisted of loose brown slightly silty gravelly sand.
- Beach deposits were observed in all windowless sample boreholes from a minimum depth of 0.20m.bgl to a maximum depth of 2.00m. Beach deposits generally consisted of loose to medium, dense brown/grey sand.
- Glacial till was observed in all windowless sample boreholes from a minimum depth of 0.70m.bgl to a maximum depth of 5.45m.bgl.

Infiltration tests have been undertaken as part of the intrusive ground investigation undertaken by GroundSolve Ltd in January 2023. The tests show that all of the trial pits failed the first cycle, as the water failed to percolate through the underlying superficial deposits.

Groundwater was encountered within all of the exploratory hole locations within either the Made Ground or wind-blown sand, at depths between 0.60m.bgl and 1.00m.bgl.

## Hydrogeology

According to NRW's Aquifer Designation data, obtained from the BGS GeoIndex online mapping [accessed February 2023], the blown sand is classified as a Secondary A Aquifer. Secondary A aquifers are 'permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as minor aquifers.

The underlying Kinnerton Sandstone Formation is classified as a Principal Aquifer. Principal Aquifers are layers of rock or drift deposits that have high intergranular and/or fracture permeability — meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale.

#### **Local Drainage**

Public sewer records have been obtained from DCWW and are included in Appendix E. The DCWW sewer records show that there is a 300mm public combined sewer originating in the eastern extent of the site



flowing east. There are also public foul and surface water sewers serving residential properties in Y Gorlan to the north-west of the site. The public foul and surface water sewers in Y Gorlan discharge to a public combined sewer in Churton Road immediately north of the site.

A GPR utility survey was undertaken by PM Surveys UK Ltd in August 2022 and is included in Appendix F. The GPR survey shows that foul flows from the site drain to the public combined sewer in the eastern extent of the site. The GPR survey identifies surface water connections into the foul drains on site, indicating that the drainage system is combined.

A drainage connectivity survey was undertaken by Invek Surveys in June 2023 and is included in Appendix G. The drainage survey confirms that foul and surface water flows from the site drain to the 300mm public combined sewer in the eastern extent of the site. The public combined manhole in the eastern extent of the site has an identified downstream invert level of 5.08m AOD.

## **Development Proposals**

The proposal is for a residential re-development to include the demolition of 59no. existing dwellings and the erection of 38no. dwellings with associated access roads, parking and gardens. A proposed development plan is included in Appendix H.

The proposed development will include hardstanding areas in the form of residential buildings, parking and access. The proposed new hardstanding areas will cover approximately 4,930m<sup>2</sup>. Measurements have been taken from a PDF copy of the 'Site Layout' (MEH-TACP-PS-ST-DR-A-701) and are approximate only.

## **Policy Context**

The Denbighshire County Council Local Development Plan contains the following policies relating to water management and drainage:

### 'Policy VOE 6 - Water Management.

All development will be required to incorporate water conservation measures, where practicable. Major development proposals (greater than 1,000 sqm floorspace or 10 dwellings) should be accompanied by a Water Conservation Statement.

All development will be required to eliminate or reduce surface water run-off from the site, where practicable. The run-off rates from the site should maintain or reduce pre-development rates.

#### **Justification**

The use of Sustainable Drainage Systems (SuDS) to manage surface water flows can also be an important tool in minimising flood risk by increasing permeable surfaces in an area which allows water to seep into the



ground rather than running off into the drainage system. The effective use of permeable surfaces, soakaways and water storage areas should be incorporated in all new development where technically possible. SuDS can also reduce the impact of diffuse pollution from run-off and flooding securing environmental, biodiversity and aesthetic benefits. Early consideration of SuDS is required in order that a range of techniques can be considered and developers are encouraged to enter into early discussions with the Council.'

### Consultation

A consultation request was submitted to the SAB in December 2022. In their response (Appendix I), the SAB have stated that:

'We would have no objections in principle to your intention of discharging the surface water from the site into the public combined sewer, providing the following conditions are met:

- 1. Evidence that the hierarchy has been followed and that you have explored and exhausted alternative surface water drainage options.
- 2. Confirmation from Welsh Water that they are happy to accept flows from the site.'

A pre-development enquiry request was submitted to DCWW in November 2022. In their response (Appendix E), DCWW have stated:

#### 'Public Sewerage Network

The proposed development site is located in the immediate vicinity of a separate sewerage system, comprising combined, foul and surface water public sewers, which drains to Kimmel Bay Wastewater Treatment Works (WwTW).

#### **Asset Protection**

This site is crossed by public watermains with their approximate position being marked on the attached Statutory Public Sewer Record. In accordance with the Water Industry Act 1991, Dŵr Cymru Welsh Water requires access to its apparatus at all times in order to carry out maintenance and repairs.

No part of any building will be permitted within the protection zone of the public watermains measured 3 metres either side of the centreline of the 100mm public watermain.

Our strong recommendation is that your site layout takes into account the location of the assets crossing the site and should be referred to in any master-planning exercises or site layout plans submitted as part of any subsequent planning application.

#### Surface Water Drainage

As of 7<sup>th</sup> January 2019, this proposed development is subject to Schedule 3 of the Flood and Water Management Act 2010. The development therefore requires approval of Sustainable Drainage Systems (SuDS) features, in accordance with the 'Statutory standards for sustainable drainage systems – designing, constructing, operating and maintaining surface water drainage systems'. As highlighted in these standards, the developer is required to explore and fully exhaust all surface water drainage options in accordance with



a hierarchy which states that discharge to a combined sewer shall only be made as a last resort. Disposal should be made through the hierarchical approach, preferring infiltration and, where infiltration is not possible, disposal to a surface water drainage body in liaison with the Land Drainage Authority and/or Natural Resources Wales.

It is therefore recommended that the developer consult with Denbighshire Council, as the determining SuDS Approval Body (SAB), in relation to their proposals for SuDS features. Please note, DCWW is a statutory consultee to the SAB application process and will provide comments to any SuDS proposals by response to SAB consultation.

In addition, please note that no highway or land drainage run-off will be permitted to discharge directly or indirectly into the public sewerage system.

#### Foul Water Drainage

We have considered the impact of foul flows generated by the proposed development and concluded that flows can be accommodated within the public sewerage system. We advise that the flows should be connected to the combined sewer at or downstream of manholes SJ01814602, located to the north-west.

No problems are envisaged with the Waste Water Treatment Works for the treatment of domestic discharges from this site.'

Further discussion with DCWW concluded that an agreement in principle to connect surface water flows to the public combined sewer cannot be made until the drainage hierarchy has been exhausted. To agree a connection with DCWW, ground investigations which discount the use of infiltration techniques will be required.

## **Surface Water Management**

The site comprises existing residential properties with associated access and parking. Surface water currently drains to the 300mm public combined sewer in the north-eastern extent of the site.

The proposed development will include 4,930m<sup>2</sup> of hardstanding in the form of residential properties, access and parking.

#### **Discharge Method**

Standard S1 of the Statutory Standards for SuDS sets out the following hierarchy of drainage options:

Priority Level 1: Surface water runoff is collected for use;

Priority Level 2: Surface water runoff is infiltrated to ground;

Priority Level 3: Surface water runoff is discharged to a surface water body;

Priority Level 4: Surface water runoff is discharged to a surface water sewer, highway drain, or



another drainage system;

Priority Level 5: Surface water runoff is discharged to a combined sewer.

#### Priority Level 1: Surface water runoff collected for use

In line with section G1.4 of the Statutory Standards for SuDS, rainwater harvesting is not proposed for this site as:

- 1. There is no foreseeable need to harvest water at the site as DCWW water resources and drought management plans do not identify potential stresses on mains water supplies;
- 2. The use of rainwater harvesting is not a viable/ cost-effective part of the solution for managing surface water runoff on the site, taking account of the potential water supply benefits of such a system.

With regards to point 2 above, section G1.6 of the Statutory Standards for SuDS states that; in most cases, rainwater harvesting alone will not be adequate to deal with the site drainage and provision will be required for an overflow to a Level 2 or lower priority runoff destination. As downstream provision of attenuation storage will be required to accommodate for rainwater harvesting system overflows, rainwater harvesting is not considered a cost-effective solution for managing surface water runoff.

#### Priority Level 2: Surface water runoff is infiltrated to ground

The next consideration for the disposal of surface water is infiltration (soakaways). As described above, the site is underlain by superficial deposits of blown sand which is underlain by Kinnerton Sandstone Formation.

Anecdotal information from the Client suggests a high groundwater table is present locally. Therefore, the use of infiltration techniques may not be feasible.

Soakaway tests have been carried out by GroundSolve Ltd in January 2023 and are included in Appendix J. The tests show that all of the trial pits failed the first cycle, as the water failed to percolate through the underlying superficial deposits. It can therefore be concluded that infiltration techniques such as soakaways will not be suitable for the discharge of surface water runoff.

#### Priority Level 3: Surface water runoff is discharged to a surface water body.

As infiltration is not suitable, a connection to a watercourse is the next consideration. The nearest watercourse is The Cut which is located approximately 260m east of the site. The site is separated from The Cut by third party, urbanised land. A connection to a watercourse is therefore not a feasible option.

## Priority Level 4: Discharge to a surface water sewer or highway drain

Where disposal of surface water to a watercourse is not possible, a connection to the public surface water sewer system is the next consideration. There is a 150mm public surface water sewer located immediately north-west of the site within Y Gorlan residential area. The 150mm public surface water sewer discharges to a public combined sewer system within Churton Road. There are no surface water sewers in the vicinity of the site (which do not connect into the public combined sewer system) which form a suitable point of connection.



#### Priority Level 5: Surface water runoff is discharged to a combined sewer

There is a 300mm public combined sewer located within the north-eastern extent of the site. Surface water flows from the site currently connect into the public combined sewer. A connection to this sewer appears to be a feasible option.

The drainage connectivity survey (Appendix G) identifies that public combined manhole SJ01815601 in the north-eastern extent of the site has a cover level of 6.55m AOD and an invert level of 5.08m AOD. A gravity connection appears feasible subject to utilising shallow depth attenuation storage features.

### **Proposed Discharge Rate**

In order to establish the proposed discharge rate, the greenfield runoff rates and existing brownfield runoff rates have been estimated.

Greenfield runoff rates have been estimated using the Revitalised Flood Hydrograph Model (ReFH2) method. A summary of the greenfield runoff rates for a range of events is provided as Appendix K. The existing 1 in 1 year greenfield runoff rate for the 0.89ha development site is 0.58 l/s.

Existing brownfield runoff rates have been estimated using the modified rational method Q=CiA, whereby:

- Q is the peak discharge (I/s);
- C is the dimensionless coefficient (2.78);
- i is the average rainfall intensity derived from FEH point data for a 6 hour storm event;
- A is the existing contributing drainage area (0.3428 ha).

A summary of the existing brownfield runoff rates is provided in Table 1.

Table 1 – Existing Brownfield Runoff Rates

Storm Event (Year)	Rainfall Intensity (mm)	Runoff Rate (I/s)
1	18.67	17.79
30	55.26	52.66
100	73.78	70.31

In relation to surface water discharge for previously developed sites, the Statutory Standards for SuDS states that 'betterment of at least 30% should be considered as a minimum requirement'.

Restricting discharge rates to the greenfield runoff rate is not considered feasible for this site. An initial review of attenuation storage volumes based on a limited discharge rate of 2 l/s (as close to the greenfield rate as practical) shows that attenuation storage depths would be in excess of 2m and would result in a requirement for a pumped solution. A pumped discharge is not in line with the principles set out in the Statutory standards



for SuDS.

As stipulated by Dŵr Cymru Welsh Water (Appendix E), a discharge rate of 5 l/s applies for a connection to the public combined sewer. A discharge rate of 5 l/s is proposed and provides 71.9% betterment on the existing 1 in 1 year brownfield runoff rate.

#### **Attenuation Storage**

In order to achieve a discharge rate of 5 l/s, attenuation storage will be required. In order to facilitate gravity drainage, attenuation storage will be distributed across the site. 2no separate attenuation storage features are proposed, one in the eastern extent of the site and one in the western extent of the site. A plan showing the contributing drainage areas into each attenuation feature is provided as Appendix L.

The proposed discharge rate will be split between the 2 drainage areas. Drainage Area 1 (eastern extent of the site) will have a limited discharge rate of 3.6 l/s. Drainage Area 2 (western extent of the site) will have a limited discharge rate of 1.4 l/s.

Attenuation storage estimates have been provided using MicroDrainage and are included in Appendix M.

#### Drainage Area 1

An estimated storage volume of 305m³ will be required to accommodate the 1 in 100 year plus 40% Climate Change (CC) event. The storage estimate is based on a discharge rate of 3.6 l/s, storage within a tank or pond structure, an impermeable drainage area of 3,540m², a design head of 1m and hydro-brake flow control.

#### **Drainage Area 2**

An estimated storage volume of 121m<sup>3</sup> will be required to accommodate the 1 in 100 year plus 40% CC event. The storage estimate is based on a discharge rate of 1.4 l/s, storage within a tank or pond structure, an impermeable drainage area of 1,390m<sup>2</sup>, a design head of 1m and hydro-brake flow control.

#### **Sustainable Drainage Systems**

Attenuation storage will be provided in the form of the sub-grade material of permeable surfaced access roads (non-adopted roads).

#### Drainage Area 1

To limit the overall sub-grade depth as to facilitate gravity drainage, the sub-grade in Drainage Area 1 will be formed from a combination of geo-cellular storm crates and clean stone. As to prevent groundwater ingress, the sub-grade will be lined with an impermeable geotextile. Groundwater floatation calculations will also be required at the detailed design stage to ensure suitable weight of cover over the geo-cellular storage crates.

Based on a permeable surfaced access road area of approximately 537m<sup>2</sup>, a stone sub-grade depth of 0.16m underlain by a geo-cellular sub-grade of 0.55m in depth would provide 306.4m<sup>3</sup> of storage, sufficient to accommodate the 1 in 100 year plus 40% CC event.

The 0.16m deep stone sub-grade, with a void ratio of 30%, would provide 25.8m<sup>3</sup> of attenuation storage. The



0.55m geo-cellular sub-grade depth, with a void ratio of 95%, would provide 280.6m<sup>3</sup> of attenuation storage.

#### **Drainage Area 2**

Based on a permeable surfaced access road area of approximately 707m<sup>2</sup>, a 0.58m deep sub-grade depth and a void ratio of 30% (applicable to stone aggregate), there is potential to accommodate 123m<sup>3</sup> of attenuation storage, sufficient to accommodate the 1 ln 100 year plus 40% CC event.

A map showing the extents of the drainage areas is included as Appendix L.

#### **Rain Gardens**

In addition to the permeable surfacing (which will provide attenuation storage) a rain garden is proposed to provide water quality, amenity and biodiversity benefits. A rain garden is proposed adjacent to the site entrance and will accommodate runoff from the adjacent adopted access road. The rain garden will be fitted with an overflow to direct runoff to the downstream attenuation storage feature.

#### **Concept Surface Water Drainage Scheme**

Surface water runoff will be discharged to the public combined sewer in the eastern extent of the site at a limited rate of 5 l/s which provides 71.9% betterment over the 1 in 1 year brownfield runoff rate. Attenuation will be provided in the form of the sub-grade of the permeable surfaced access roads (excluding the section of adopted road) to accommodate the 1 in 100 year plus 40% CC event. A rain garden is also proposed in the north-western extent of the site.

A Concept Drainage Sketch is included in Appendix N.

The proposed surface water drainage scheme will ensure no increase in runoff over the lifetime of the development.

#### **Exceedance Event**

Storage will be provided for the 1 in 100 year plus 40% CC event. Storm events in excess of the 1 in 100 year plus 40% CC event should be permitted to produce temporary shallow depth flooding within the proposed access roads and landscaped areas. Finished floor levels will be set at a minimum of 150mm above surrounding ground levels ensuring exceedance flooding will not affect the buildings.

#### **Surface Water Treatment**

The Statutory Standards for SuDS sets out the following guidance for surface water treatment;

#### S3 - Surface water quality management

Treatment for surface water runoff should be provided to prevent negative impacts on the receiving water quality and/or protect downstream drainage systems, including sewers.

In accordance with the CIRIA C753 publication 'The SuDS Manual' (2015), residential roofs have a 'very low' pollution hazard level, with low traffic roads classified as having a 'low' pollution hazard level. Table 2 shows the pollution hazard indices for each land use.



**Table 2 – Pollution Hazard Indices** 

Land Use	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Residential Roofs	Very Low	0.2	0.2	0.05
Low Traffic Roads	Low	0.5	0.4	0.4

Table extract taken from the CIRIA C753 publication 'The SuDS Manual' – Table 26.2

Runoff from roofs and roads will be directed to the proposed permeable surfacing. Table 3 demonstrates that permeable surfacing provides sufficient treatment.

Table 3 – SuDS Mitigation Indices

	Mitigation Indices							
Type of SuDS	Total Suspended Solids (TSS)	Metals	Hydrocarbons					
Permeable Pavement	0.7	0.6	0.7					

Table extract taken from the CIRIA C753 publication 'The SuDS Manual' - Table 26.3

#### **Amenity**

The Statutory Standards for SuDS provide the following guidance in relation to Standard S4 – Amenity:

'The design of the surface water management system should maximise amenity benefits.'

The proposed development will include permeable surfacing and a rain garden which will maximise the amenity value of the proposed drainage system.

## **Biodiversity**

The Statutory Standards for SuDS provide the following guidance in relation to Standard S5 – Biodiversity:

'The design of the surface water management system should maximise biodiversity benefits.'

The proposed rain garden will maximise the biodiversity value of the proposed drainage system.



<sup>\*</sup> Indices values range from 0-1.

## **Construction, Operation and Maintenance**

Standard S6 of the Statutory Standards for SuDS states;

#### S6 – Design of drainage for Construction, Operation and Maintenance

1) All elements of the surface water drainage system should be designed so that they can be constructed easily, safely, cost-effectively, in a timely manner, and with the aim of minimising the use of scarce resources and embedded carbon (energy).

- All elements of the surface water drainage system should be designed to ensure maintenance and operation can be undertaken (by the relevant responsible body) easily, safely, cost-effectively, in a timely manner, and with the aim of minimising the use of scarce resources and embedded carbon (energy).
- The surface water drainage system should be designed to ensure structural integrity of all elements under anticipated loading conditions over the design life of the development site, taking into account the requirement for reasonable levels of maintenance.

All drainage systems will be readily accessible for maintenance access. The drainage system will be offered for adoption to the SAB who will then be responsible for maintenance. Alternatively, the drainage system will be managed and maintained by the site owner (Denbighshire County Council).

Maintenance schedules for an attenuation tank (geo-cellular storage) and permeable paving are included in Appendix O.

Shallow groundwater is present on site. As to prevent groundwater ingress, the sub-grade of the drainage system will be lined with an impermeable geotextile. Groundwater floatation calculations will also be required at the detailed design stage to ensure suitable weight of cover over the geo-cellular storage crates.

## **Foul Drainage**

Correspondence from DCWW (Appendix E) states that:

#### 'Foul Water Drainage - Sewerage Network

We have considered the impact of foul flows generated by the proposed development and concluded that flows can be accommodated within the public sewerage system. We advise that the flows should be connected to the combined sewer at or downstream of manholes SJ01814602, located to the north west.'

The connection point suggested by DCWW is located in Churton Road near the site access. In order to facilitate gravity drainage, a connection to public combined manhole SJ01815601 in the eastern extent of the site, as per the existing situation, is proposed. Manhole SJ01815601 has an invert level of 5.08m AOD and a gravity connection is considered feasible. Manhole SJ01814602 in Churton Road has an identified invert level of 5.2m AOD.



### **Conclusions**

The proposal is for a residential re-development to include the demolition of 59no. existing dwellings and the erection of 38no. dwellings with associated access roads, parking and gardens.

All methods of surface water discharge have been assessed. Where soakaways are not possible, surface water runoff will be discharged to the public combined sewer in the eastern extent of the site at a limited rate of 5l/s which provides 71.9% betterment over the 1 in 1 year brownfield runoff rate. A gravity connection appears feasible subject to utilising shallow depth attenuation storage features.

Attenuation will be provided in the form of the sub-grade of the permeable surfaced access roads (excluding the section of adopted road) to accommodate the 1 in 100 year plus 40% CC event. A rain garden is also proposed in the north-western extent of the site. The sub-grade will be formed from a combination of stone and geo-cellular storage.

DCWW have confirmed that foul flows can discharge to the public combined sewer system. A gravity connection can be achieved.

A Concept Designer's Risk Assessment (cDRA) has been prepared to inform future designers of any identified hazards associated with the scheme. The cDRA has been included in Appendix P.

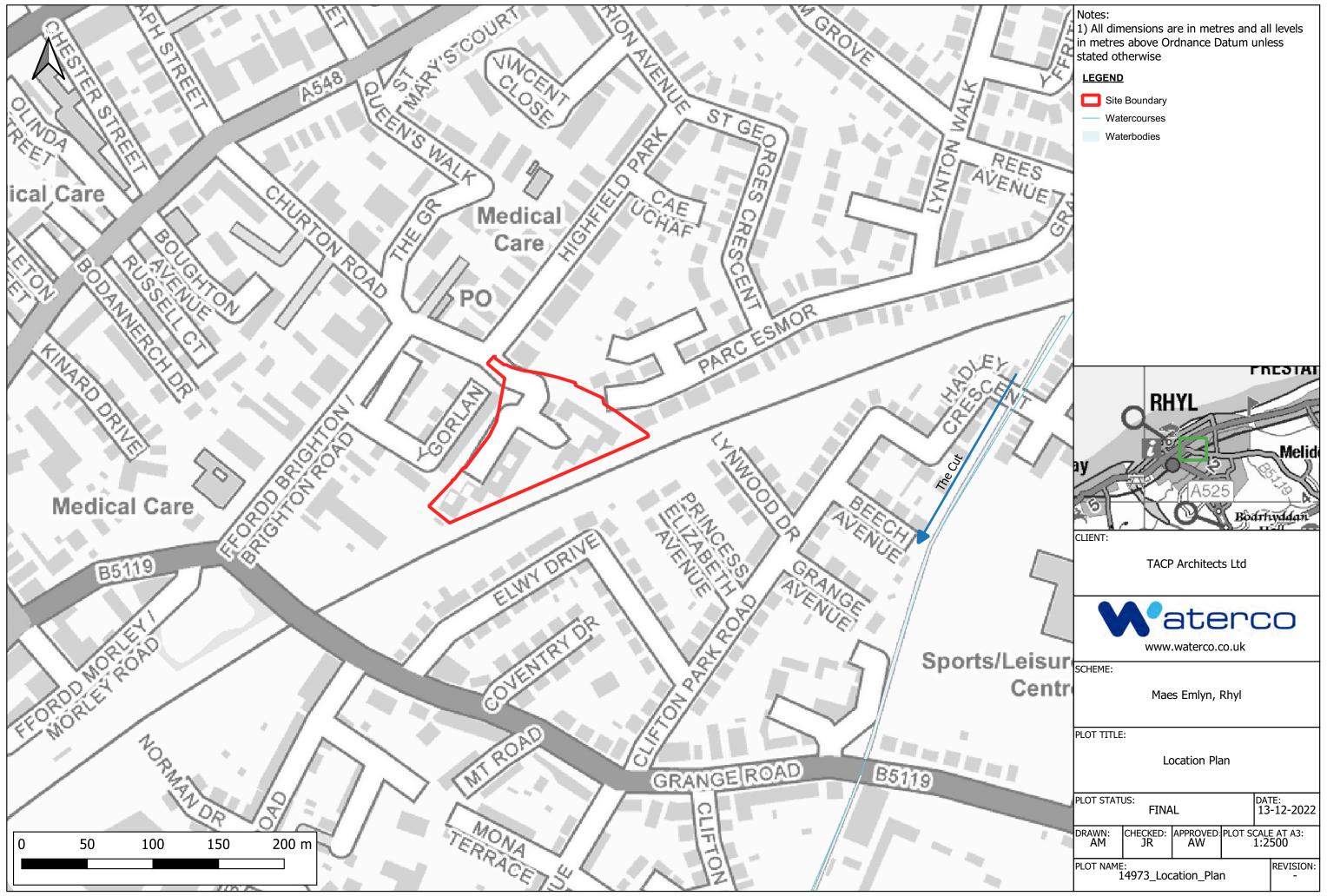
#### Recommendations

- 1. Submit this Drainage Strategy to the Planning Authority in support of the Planning Application.
- 2. Verify the attenuation volumes included in this report when undertaking detailed drainage design.



# Appendix A Location Plan & Aerial Image

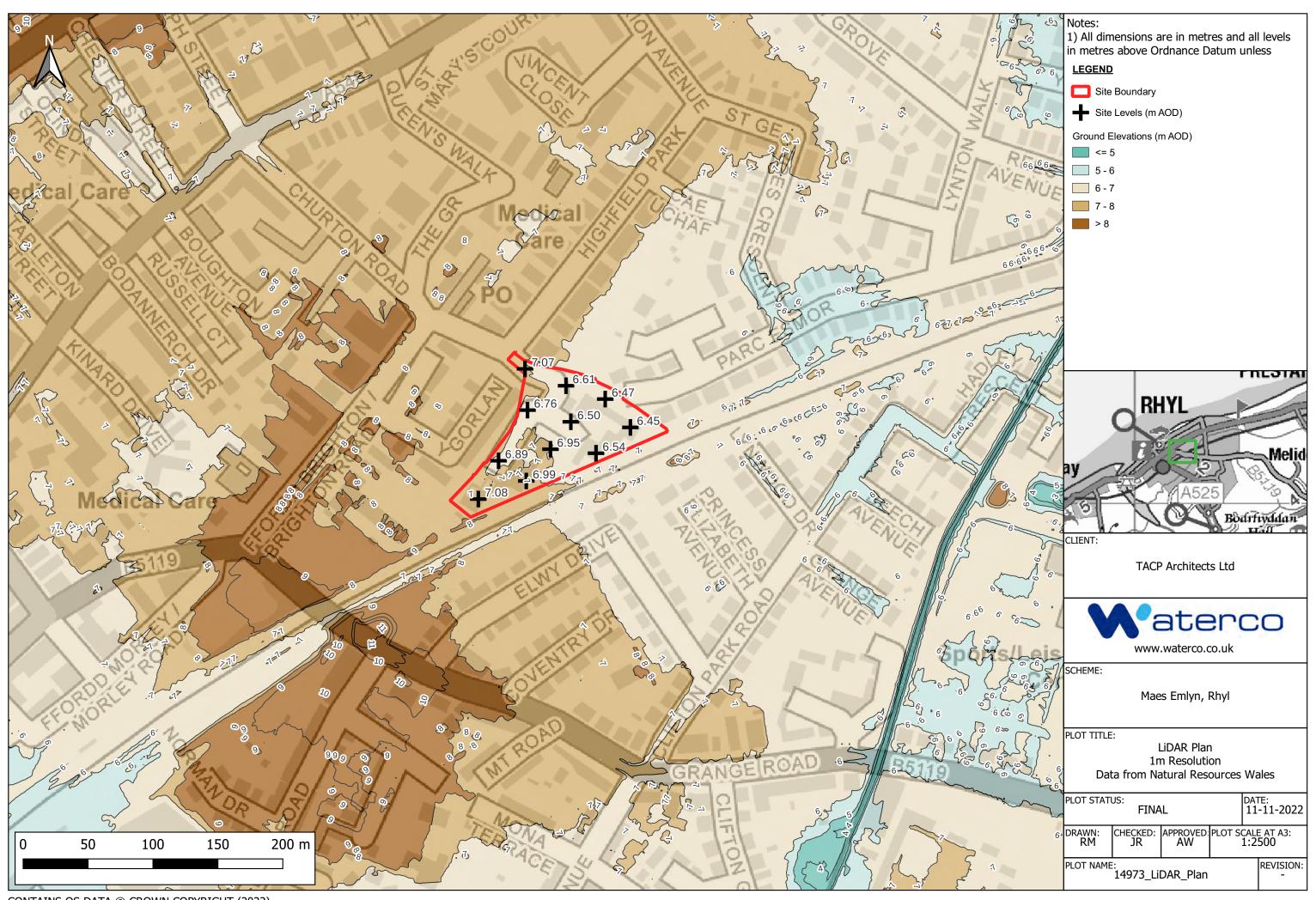






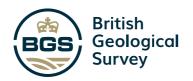
# Appendix B Topographical Information





# Appendix C BGS Borehole Record





## BGS ID: 686119 : BGS Reference: SJ08SW10 British National Grid (27700) : 301300,381900

Report an issue with this borehole

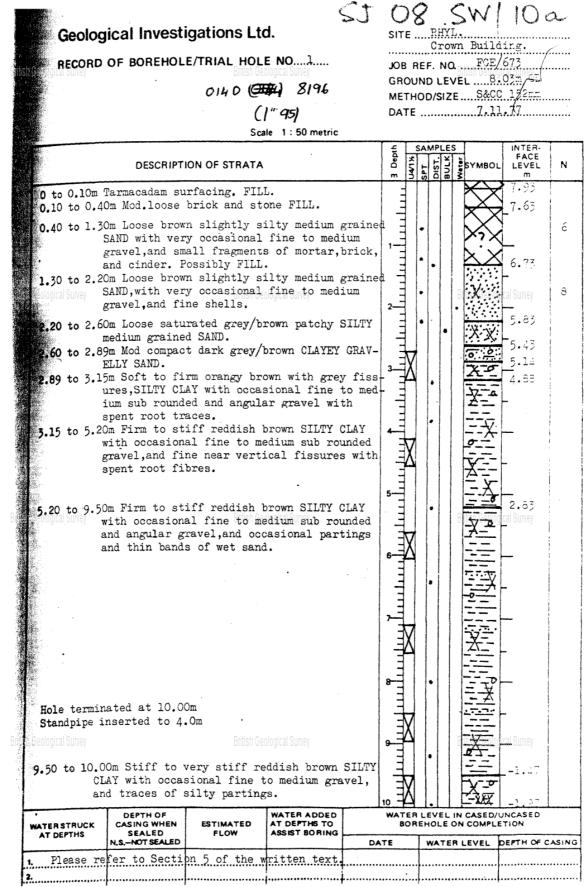
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(1"95)

ECORD OF BOREHOLE/TRIAL HOLE NO....2....

0140 8194

SITE RHYL.
Crown Building.

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METHOD/SIZE S&CC 152=00
DATE 8-11.11.77

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	artings.			1	=				五大		
ritish Geological I					Ξ				`		
	m Firm reddis				$\exists$				X-0		
	ILTY CLAY with ravel.	tn traces of	line sub rou	inded	6-	7				-	
	m Firm reddis	ah hasum CTI	nv ctav mi+h	+	=\_	V				1.37	
	f fine gravel		ri Chai With	traces	=					-	
-	a a a a a a a a a a a a a a a a a a a				$\exists$						
				[	7		•	-	X		
					7				0		
•					XΕ			-	EXE		
•					4	4			==		
to 9.25	m Stiff reddi	ish brown SII	LTY laminated	CLAY	•		•		72	=0.38	
,					Е				XI		
					=		-		X		
			British Geological Surv	84	• <del>=</del>				X-V	<del>ritis</del> h Geological	
	Om Dense satu	rated orev/			ΧE					-1.53	
	ed SAND with				7	Y		•	ο σ	_	.1.3
	well graded S	SAND and fine			$\exists$				0.0		
	rounded GRAVI	EL.	T	1	10 7		Ц		19:00	11100000	
STRUCK	DEPTH OF CASING WHEN	ESTIMATED	AT DEPTHS TO						N CASED/ N COMPL	UNCASED ETION	
DEPTHS	SEALED N.SNOT SEALED	FLOW	ASSIST BORING	DAT	rE	T	WA.	TER	LEVEL	DEPTH OF CA	ASING
lease re	fer to Section	on 5 of the v	vritten text.			T					
	1										
						1				I	

SI 08 SW (10
SITE BHYL.
Crown Building.
OB REF. NO FGE/67 social Survey
GROUND LEVEL7.727. CD
METHOD/SIZE
DATE

Scale 1:50 metric							SAMPLES					INTER-	
	DESCRIPTI	ON OF STRATA			Depth		SPT	TST	BULK	ater	SYMBOL	FACE	N
00 to 10.	50m Dense sat	urated grey/	brown well g	raded		3	0	9	8	3	0:00	m	-
	SAND and C					=					0.0.00	-2.76	
50 to 12.	00m Firm to s						2				7-0		
		mal fine to gravel, and f			11-	7	1		•			-	
	silty sand.					=							
												iah Geological S	uncy
00 to 13.	50m Firm to s	tiff reddish	brown SILTY	lamin	12-	=	•		•			4.28	42
		th bands of				$\exists$					X		
	ium grained	ands of wet SAND.	silty line to	mea-		$\exists$				-	$\Rightarrow$		
					13	Ę					<del>X</del> =		1
						=					=X		
50 to 16.0	00m Firm to s	tiff reddish ted CLAY wit			-	Ē		•			77-	-5.78	26
		silty fine			14-	$\exists$					<del>*</del>	_	
						=					<u> </u>		
						-	1				¥Ξ	-	
					45-	3					三文.		
					15-	=					弘	ish Geological S	
						=					X.X.	-	
										1	3		
	ated at 16.00 nserted to 12				16-	=		•		ľ		-8.28	
andpipe in	nserted to 12	• Ош			-	=						_	
						=							
					17-	=						_	
					-	╡						_	
						3							
					18-	=						-	
						=							
						Ⅎ						sh Geological S	
					19-	╡						-	
						=							
					-	7							
	050711.05		WATER ADDED		20	1				1	CASED	INCASSO	
ERSTRUCK DEPTHS	DEPTH OF CASING WHEN SEALED	ESTIMATED FLOW	AT DEPTHS TO ASSIST BORING		BOREHOLE ON COMP								
	N.SNOT SEALED			DA	AT E		4	WA	TE	RL	EVEL	DEPTH OF C	ASIN
	.]							••••	••••	••••			

SJ 08 SW / (0 c. RHYL. Crown Building.

RECORD OF BOREHOLE/TRIAL HOLE NO....4A...

JOB REF. NO. FGE/673 0138 8194 GROUND LEVEL .... 7.85m OD METHOD/SIZE \$&CC 152mm DATE .....9-11.11.77

		DESCRIPTIO	N OF STRATA			3 Depth		DIST. W	SYMBOL	INTER- FACE LEVEL m	N
	0 to 0.40m	Concrete over	stone FILL.					П			
		10m Loose to mod grained sand wi of clinker, brid	d compact brith occasion	al small fra	dium gments	2	•			7.45	7
		Probably FILL.				` =			13		
British Geo	logical Survey		British Geolog			=		•		arrey	8
	2.10 to 3.	30m Very loose medium grained	wet grey/bro	own SILTY fin	e to	2		•		5.75	2
	+4'55		51115 172 011	.14005 01 014	J •			•	X	-	3
						3		•	± X	_	
	1435	50m Firm grey/b pockets of gre 50m Very stiff	yish green S	SILT.		=	M	-	X X	4.55 - 4.35	
	J.50 to 1.	brown with gre CLAY with occas rounded and an	y fissures a sional fine	and streaks,S to medium su	ILTY	4-	Δ	•		_	
				•		5-				_	
	lògical Survey .					=	Å				
						6-			臺	_	
						=	V		.D	-	
						7		•		_	
	7.50 to 10.	00m Soft to fin	rm reddish b	rown partial	ly				===°	- 0.35	
	1	gravel.	JII OLMI WI	in traces or		8				_	
			•			Ξ	Å			_	
	Spical Survey		British Geolog			9		1	屋	Bulley	
						-					
						<u> </u>	X		差	- i 1 *	
	MATER STRUCK	DEPTH OF CASING WHEN	ESTIMATED	WATER ADDED					L IN CASED/		-
	AT DEPTHS	SEALED N.SNOT SEALED	FLOW	ASSIST BORING	DA	TE				DEPTH OF	CASING
	Please	refer to Section	n 5 of the y	ritten text.					•••••		

RECORD OF BOREHOLE/TRIAL HOLE NO....4A... continued.

SJ	08	SW	0 (
SIT	E RHYL	<b></b>	 

Crown Building.

JOB REF. NO. FGE/673

GROUND LEVEL 7.85m OD

METHOD/SIZE.....

		Sca	le 1:50 metric								
15 5 14 1				1	Depth			LES		INTER-	
	DESCRIPTI	ON OF STRATA			å	U4/1%	TSIC	BULK	SYMBOL	LEVEL	N
-2.95	gravel.  6.40m Compact silty fine to	LTY CLAY with saturated gre o coarse SAN	n occasional	ntly	=				0000	-2.95	13
Seological Survey	coarse sub I		logical Survey		12—1111			•	000000	tal Survey	15
					<b>13</b>			•	0000	<b>-5.</b> 55	
13.40 to 15 -7.95		to coarse SAN	ngy brown sli D,and fine t angular GRAVE	<b>&gt;</b>	diniland			•	000		30
Geological Survey		· British Geo	logical Survey	1	ts-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1			•	000000	cal Survey	43
15.80 to 17			ne to medium s	sub 1	<u></u>	Z	•		X-0	-7.95 - -	
17.50 to 17	.70m Stiff gre	ey SILT			류				<del></del>	9.65	
17.70 to 18		SAND, and fi	slightly sil		* 11 T	1	•			-9.85 -	
0,8,70 to 20	.00m Firm to s		brown VERY S	.			•			-10.85 cal Survey	29
ATER STRUCK AT DEPTHS	DEPTH OF CASING WHEN	ESTIMATED	WATER ADDED AT DEPTHS TO ASSIST BORING						IN CASED/L		
	SEALED	FLOW									

British Geological SIRECORD OF BOREHOLE/TRIAL HOLE NO...4A... continued.

SITE RHYL.
Crown Building.
JOB REF. NO FGE/673
GROUND LEVEL 7.85m OD
METHOD/SIZE
DATE

DESCRIPTION OF STRATA  20.00 to 23.50m Stiff reddish brown VERY SILTY CLAY, with occasional fine gravel, silt and sand partings (wet).  23.50 to 25.00m Very stiff reddish brown VERY SILTY CLAY with occasional fine gravel, and thin wet sand partings.  23.50 to 25.00m Very stiff reddish brown VERY SILTY CLAY with occasional fine gravel, and thin wet sand partings.  24.				ale 1 - 50 metric							
20.00 to 25.50m Stiff reddish brown VERY SILTY CLAY, with occasional fine gravel, silt and sand partings (wet).  23,50 to 25.00m Very stiff reddish brown VERY SILTY CLAY with occasional fine gravel, and thin wet sand partings.  24		DESCRIPTION	ON OF STRATA		Depth			1	SYMBOL	FACE	N
23.50 to 25.00m Very stiff reddish brown VERY SILITY CLAY with occasional fine gravel, and thin wet sand partings.  23.50 to 25.00m Very stiff reddish brown VERY SILITY CLAY with occasional fine gravel, and thin wet sand partings.  24.50  25.50  Hole terminated at 25.00m  26.51  27.15  28.50  29.50  20	W	5.50m Stiff revith occasiona	ddish brown	VERY SILTY C	LAY,	3	DIS	3 \$	X=	m	
23,50 to 25,00m Very stiff reddish brown VERY SILTY CLAY with occasional fine gravel, and thin wet sand partings.  Hole terminated at 25,00m Emish Geological Surrey  Emish Geological Surrey  Emish Geological Surrey  Depth or Casing when scaled Nas-Motor Topprins Nasa Pauled Nasist Borning  WATER ADDED NAS-MOTOR DEPTH OF CASING WHEN SCALED NAS-MOTOR TOP PLAN TO ASSIST BORNING  DATE WATER LEVEL DEPTH OF CASING WATER ADDED ASSIST BORNING  DATE WATER LEVEL DEPTH OF CASING	·	ertings(wet).	British Geolog				•	15,215,3 15,215,3 10,210,0 10,0	SE SE	urvey	
CLAY with occasional fine gravel, and thin wet sand partings.  Hole terminated at 25.00m  British Geological Survey  Depth Of Casino Water Level in CaseD/JunicaseD Bookehole On Communication Co	07 EO +o 25	. OOm Verw sti	ff reddish l	hrown VERY SI	23		•			15.65	
British Geological Survey  British Geological Su		CLAY with occa wet sand part:	asional fine	e gravel, and	thin 24	X	•			-	53
British Geological Survey  British Geological Survey  29  British Geological Survey  30  WATER ADDED  AT DEPTHS TO ASSIST BORING  DATE  WATER LEVEL IN CASED/UNCASED BOREHOLE ON COMPLETION  ASSIST BORING  DATE  WATER LEVEL DEPTH OF CASING		ated at 25.00r					•	C.T.C.) C.T.C.) C.T.C.) C.T.C.)	Geological (	1	
DEPTH OF CASING WHEN SEALED N.SNOTSEALED ESTIMATED ASSIST BORING DATE WATER LEVEL IN CASED/UNCASED BOREHOLE ON COMPLETION  MATER STRUCK AT DEPTHS TO ASSIST BORING DATE WATER LEVEL DEPTH OF CASING					111111111111111111111111111111111111111				•	-	
DEPTH OF CASING WHEN SEALED N.SNOT SEALED  DEPTH OF CASING WHEN SEALED  DEPTH OF CASING WHEN SEALED  DEPTH OF CASING WATER ADDED AT DEPTHS TO BOREHOLE ON COMPLETION  DATE  WATER LEVEL IN CASED/UNCASED BOREHOLE ON COMPLETION  DATE  WATER LEVEL DEPTH OF CASING					Inflin			EUC)	Geological	Sunvey	
N.SNOTSEALED DATE WATER LEVEL DEPTH OF CASING	MATER STRUCK	CASING WHEN		AT DEPTHS TO	WAT						
	AT DEPTHS			- BONING	DATE	WATER LEVEL D				DEPTH OF CASING	
		olg. a		<u> </u>	***************************************						

# Geological Investigations Ltd. (1"95) SITE ....RHYL.

ECORD OF BOREHOLE/TRIAL HOLE NO....5.....

0136 8191

Crown Building.

JOB REF. NO. FGE/673

GROUND LEVEL 7.43m OD

METHOD/SIZE S&CC 152mm

DATE 14/15.11.77

						£		MP			INTER-	T
		DESCRIPT	ION OF STRATA	A		3 Depth	X1/X	DIST.	BULK	SYMBOL	FACE LEVEL m	1
0 60	0.60m	Black TOPSOII				=				XX		
			,			=			-	X	6.83	
0.60	to 1.9	5m Loose grey	/brown mediu	um grained SA	ND wit	片 극						7
		traces of she FILL.	II, glass and	clinker. Po	ssibly	1-						
		ETIL.		•			•			2		
			· British Geolog			111111111111111111111111111111111111111				$\sim$		
	2 5	om Compact sa		/brown mediu	m	الح				XX	5.48	2.
		grained SAND.	GV	,	-	1 =						
				W OTTMY		4					- 4.93	
(0.1	io 3.00	Om Compact gr	d SAND with	Y SILTY media patches of g	um to	=			•	XX	7-77	
		black SILT.an	d traces of	organic matte	er.	3_				<b>W</b> .^.	-4.43	
				h grey fissu		1 3	V			X-0		
, W 1	. ٠٠٠ ت	and streaks,S	ILTY CLAY wi	th fine to co	arse	=	Δ			<u> </u>	- 7 7 9	
	٤	ub rounded g	ravel, and sp	ent root fib		=		•		77-	3.78 3.73	
A STATE OF THE STA		m Fine GRAVE				4-				X	_	
3.70 t	0 8.70	m Firm with	local firm to	o stiff bands	3,	I	1			근도		
				with occasion d and angular		I B	X		1	<u>-</u> -\	-	
		ravel, and the			•	===	4			===	_	
		-	British Geolog	rical Survey		=			adis	2	UIVEV	
				, - <del></del> ;		I I					-7	
						I I	4			大二		
						6				35.	-	
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	*.					7			-	2	-	
						走	7			==		
		•	`			3/	V			25	• .	
						E	]			-X-	_	
						٦			1	兰	-	
						Ξ					. }	
70 to	10.00	Om Stiff redd	ish brown VE	ERY SILTY CLA	Y with	±			١		-1.27	
erad Guiraj Revor	00	casional bro		ained wet sa		(إــو		1	1	X=-	штбў	
A	pa	artings.				7	7	•		三三		
	•					크				壶		
						Ę"				YE	-2.57	
		DEPTH OF		WATER ADDED		WATE	RLE	VE	LIN	CASED/UI		
No. 5	RUCK	CASING WHEN	ESTIMATED	AT DEPTHS TO						COMPLET		
MATERST		SEALED	FLOW	ASSIST BORING								

British Geological Survence OF BOREHOLE/TRIAL HOLE NO....5..... continued.

SJ	08 SW 10 d
	SITE RHYL.
	Crown Building.
	JOB REF. NO. FGE/673
ed.	GROUND LEVEL7.43m. OD
	METHOD/SIZE
	DATE

-	<del></del>										
	DESCRIPTI	ON OF STRATA		L Capacit		SAA			SYMBOL	FACE LEVEL m	N
10.00 to 1	5.30m Firm to with occasions and angular gr	al fine to m	h brown SILT edium sub ro	Y CLAY	X		•		X-2	-	
British Geological Survey		British Geologic		12-				2000 2000		E C Y	
		e e e e e e e e e e e e e e e e e e e		13-						_	
British Ge of Tal Survey 15.30 to 18	.20m Soft to f	British Geologic irm reddish ces of fine	brown VERY S	ILTY avel.				100000 100000 1000000	<b>美</b>		
			• .	17	X				京 宗 宗 宗 宗 宗 宗 宗 宗 宗 宗 宗 宗 宗	-	
Bmish Group invey	.50m Firm to s ith occasional nd angular gra ated at 19.50m nserted to 4.0	fine to med vel British Geologic	n brown SILTY lium sub roun alSurvey	ded 19-	X			t		-10.77 - Ney -	
WATERSTRUCK	DEPTH OF CASING WHEN	ESTIMATED	WATER ADDED	20  WA					CASED/U	JNCASED	$\neg$
AT DEPTHS	SEALED N.SNOT SEALED	FLOW	ASSIST BORING	DATE						DEPTH OF C	ASING
						Ī					
L	1										

# Appendix D Windowless Sample Logs





WS01

Project Na	ame: Maes Er	mlyn		Client: [	Denbighshi	re CC			Date: 18/01/2023					
Location:	Rhyl			Contrac	tor: Region	nal Drillin	ıg		Co-ords: E301418.00 N	381546.00				
Project No	o. : 2841			Crew N	ame: Regio	onal Drill	ing		Drilling Equipment: Prer	mier Compact 11	10			
	le Number		Type	7	Level		Logged	Ву	Scale	Page Numb				
				1		Lavial	AB		1:35	Sheet 1 of	1			
Well Stri					(m)	(m)	Legend		Stratum Description	on				
Wall Wa		mple and limited in the second	N=3 (1,1/0) N=14 (2,2/3) N=28 (4,5/6) N=28 (5,5/6) N=21 (3,4/5)	ng ts (3,1,1,1)	20m AoD Depth (m) 0.20 1.50	Level (m) 7.00 5.70	AB Legend	SILT with subrour (TOPSC Loose to SAND.)  Stiff bro (GLAC).	lark brown slightly gravelly s th rootlets. Gravel is subang ided fine to coarse of mixed DIL) o medium dense brown grey (BEACH DEPOSITS)	Slightly sandy pular to I lithologies.  I fine to coarse  Om and 5.45m	1 1 2 - 3 - 5			
Depth Base 1.00 2.00 3.00 4.00 5.45	200 100 92 79 70	Casing Depth Base 1.00 2.00	Diameter Diameter 150 150	Depth To	Depth Bε	Chiselling ase Dur	ation	Tool	Inclination and Depth Top Depth Base I	d Orientation nolination Orient	7 —			
Remarks	1													



WS02

Projec	t Name	: Maes Emlyn			Client: [	Denbighshii	e CC		Date: 18/01/2023				
Locati	on: Rhy	I			Contrac	tor: Region	al Drillir	ng		Co-ords: E	301494.00	N381588.0	)0
Projec	t No. : 2	2841			Crew N	ame: Regio	nal Drill	ing		Drilling Eq	uipment: Pr	emier Com	ipact 110
Bor	ehole N WS02			Type /LS	6.	Level 70m AoD		Logged AB	Ву		cale :35		Number et 1 of 1
Well	Water	Sample	and l	n Situ Testir	ng	Depth	Level	Legend			um Descrip		
X//X//	Strikes	Depth (m)	Туре	Resul	ts	(m)	(m)		Loose o		ightly gravell		ndv
		0.30	ES			0.40	6.30		SILT wit (TOPSC	th rootlets. DIL) o medium de	ense brown S		
	•	1.00 1.20	D SPT	N=2 (1,0/0,	1,0,1)								1 -
		2.00	SPT	N=15 (1,2/3	3,3,4,5)	1.60	5.10		·   (GLACI.	wn sandy Cl AL TILL) s <i>less sand</i>	LAY. <u>dy wit</u> h dept	h	2 -
		2.50	D SPT	N=14 (2,3/3	i,3,4,4)								3 -
		3.70	D										
		4.00	SPT	N=21 (3,4/5	5,5,5,6)				No samp	ole recover	y due to wa	ter	4 -
		5.00	SPT	N=19 (2,4/4	.,4,5,6)	5.45	1.25			End of	Borehole at 5.	.450m	5 -
										L10 011	ESTOTION AL S.		6 -
Depth 1.00 2.00 3.00 4.00 5.41	) ) ) ) 5	Diameter Dep	Casing th Base 1.00 2.00	Diameter Diameter 150 150	Depth To	pp Depth Ba	Chiselling se Dur	ation	Tool	Depth Top	Inclination Depth Base	and Orientation Inclination	Orientation



WS03

Unit 1, Charter Court Well House Barns Chester Road Bretton Flintshire CH4 0DH

Project Name: Maes E	mlyn		Client: [	Denbighshi	re CC		Date: 18/01/2023					
Location: Rhyl			Contrac	tor: Region	al Drillin	g		Co-ords: E	301525.00	N381613.0	00	
Project No. : 2841			Crew Na	ame: Regio	nal Drilli	ng		Drilling Eq	uipment: Pi	remier Com	pact 110	
Borehole Number WS03		Type /LS	6	Level 48m AoD		Logged AB	Ву	Į.	cale :35		Number et 1 of 1	
Water Sa	1	n Situ Testin		Depth	Level						etioni	
Well Strikes Depth		Result		(m)	(m)	Legend			um Descrip			
0.18	5 ES			0.15	6.33		SILT wit subrour (TOPSC	h rootlets. G ided fine to c DIL)	ravel is suba coarse of mix	ed lithologie	S	
				0.70	5.78		angular (MADE Loose to	fine to coars GROUND) o medium de	e of brick.	ly SAND. Gra		
1.20	) SPT	N=5 (0,0/0,	1,2,2)				DEPOS	ITS)			1 -	
1.40		•	, ,	1.50	4.98		Black pa ⊲odour 1. Stiff bro	ntch in sand 3 to 1.5m wn sandy Cl	with slight	hydrocarbo	on	
2.00		N=24 (3,4/5	,6,6,7)				(GLACI	AL TILL)			2 -	
3.00		N=27 (5,6/5	,6,8,8)								3 -	
4.00	) SPT	N=25 (3,4/5	,7,6,7)				Become	s slightly gi	r <u>avel</u> ly 3.8n	n to 4.2m	4 -	
5.00	) SPT	N=22 (4,5/5	,5,6,6)	- 4-	4.00						5 -	
				5.45	1.03			End of I	3orehole at 5	.450m		
											6	
Holo Diameter	Cosine	Diameter I			Chicolling			Τ	Inclination	and Orientation	7 -	
Hole Diameter	Depth Base 1.00 2.00	Diameter Diameter 150 150	Depth To	pp Depth Ba	Chiselling ise Dura	ation	Tool	Depth Top	Depth Base	Inclination	Orientation	



**WS04** 

Client: Denbighshire CC Date: 19/01/2023 Project Name: Maes Emlyn Co-ords: E301439.00 N381583.00 Location: Rhyl Contractor: Regional Drilling Project No.: 2841 Crew Name: Regional Drilling Drilling Equipment: Premier Compact 110 Borehole Number Hole Type Level Logged By Scale Page Number 6.96m AoD WS04 WLS ΑB 1:35 Sheet 1 of 1 Sample and In Situ Testing Water Depth Level Well Stratum Description Legend Strikes (m) (m) Depth (m) Type Results Tarmac 0.15 6.81 (MADE GROUND) 0.20 ES Loose black sandy angular fine to coarse 0.30 6.66 (MADE GROUND) 0.50 6.46 Loose pinkish brown sandy angular fine to coarse GRAVEL of limestone. (HARCORE) 0.70 6.26 (MADE GROUND) Loose to medium dense brown SAND. (BEACH DEPOSITS) 1.00 D Soft grey sandy CLAY. 1.20 SPT N=2 (1,0/0,0,1,1) (GLACIÁL TILL) 1.80 5.16 Stiff brown slightly sandy CLAY. (GLACIAL TILL) 2.00 SPT N=16 (2,3/3,4,4,5) 2.10 3.00 SPT N=22 (3,4/4,5,7,6) 3 3.50 D 4.00 SPT N=14 (2,3/3,3,4,4) No sample recovery due to water 5.00 SPT N=20 (3,4/5,5,5,5) 5.45 1.51 End of Borehole at 5.450m 7 Hole Diameter Chiselling Inclination and Orientation Casing Diameter Depth Base Depth Base Diameter Diameter Depth Top Depth Base Tool Depth Top Depth Base Inclination Orientation 1.00 2.00 3.00 4.00 5.45 1.00 2.00 Remarks



WS05

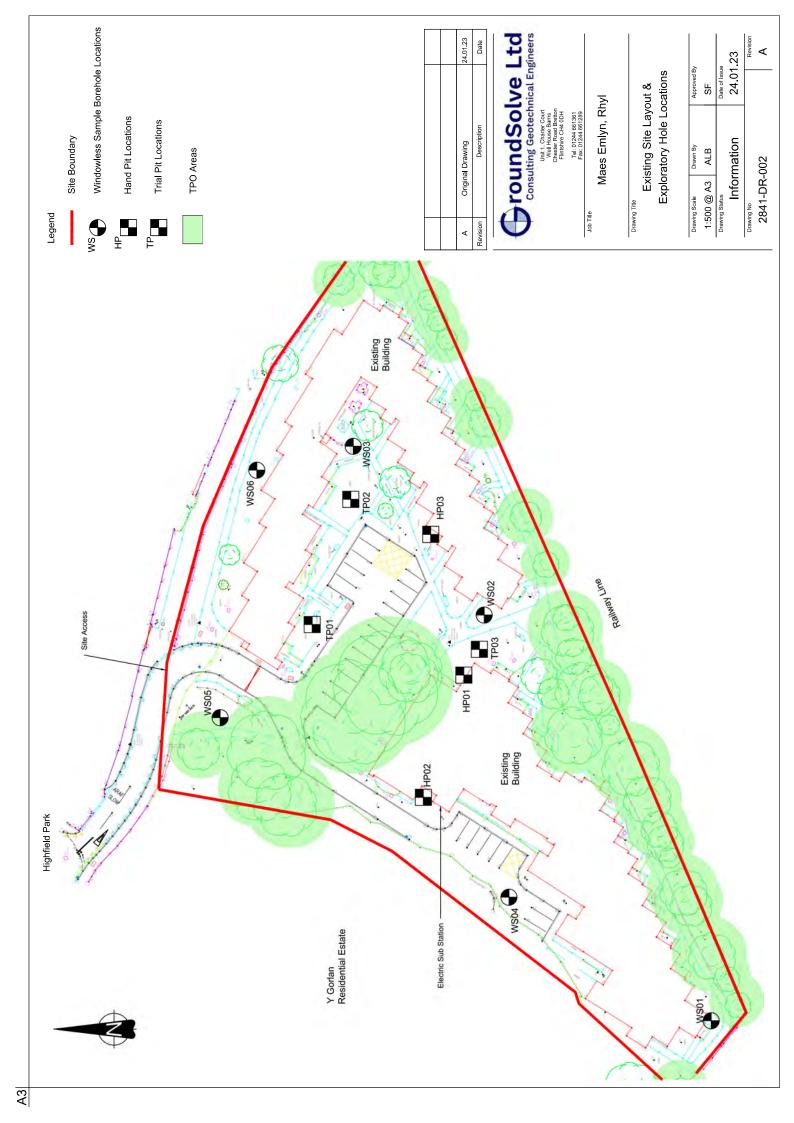
Project Name:	Maes Emlyn			Client: [	Denbighshi	re CC		Date: 19/01/2023					
Location: Rhyl				Contrac	tor: Region	nal Drillin	g		Co-ords: E	301477.00	N381641.0	00	
Project No. : 28				Crew N	ame: Regio	onal Drilli	ng		Drilling Eq	uipment: Pı	emier Com	pact 110	
Borehole Nu WS05			Type LS	7	Level 15m AoD		Logged AB	Ву	l.	cale :35		Number et 1 of 1	
Well Water Strikes	Sample	and Ir	ı Situ Testir	ng	Depth (m)	Level (m)	Legend			um Descrip	_	<u>et 1 01 1</u>	
Ourkes	Depth (m) 0.30	Type ES	Result	ts	0.40	6.75		SILT wit subrour (TOPSC	o medium de	ravel is suba oarse of mix	ingular to ed lithologie:	S. :	
	0.90	В										1 -	
	1.20	SPT	N=1 (0,1/0,	0,0,1)									
	2.00 2.10	SPT D	N=9 (1,2/1,	2,3,3)	1,90	5.25		Firm gre	s grey in co ey sandy CL/ AL TILL) s brown in	AY.	less sandy	2 -	
	3.00	SPT	N=23 (4,4/5	(,6,5,7)								3 -	
	3.90 4.00	D SPT	N=21 (3,4/4	-,5,6,6)								4 -	
	5.00	SPT	N=16 (3,3/4	.,3,5,4)	5.45	1.70			End of I	3orehole at 5	450m	5 -	
Hole Diamet	ter	Casing I	Diameter			Chiselling				Inclination	and Orientation	7 -	
Depth Base Di	iameter Dept	th Base 1.00	Diameter 150	Depth To	op Depth Ba		ation	Tool	Depth Top	Depth Base	Inclination	Orientation	
2.00 3.00 4.00 5.45	100 2 92 79 70	2.00	150										



# Windowless Sample Record

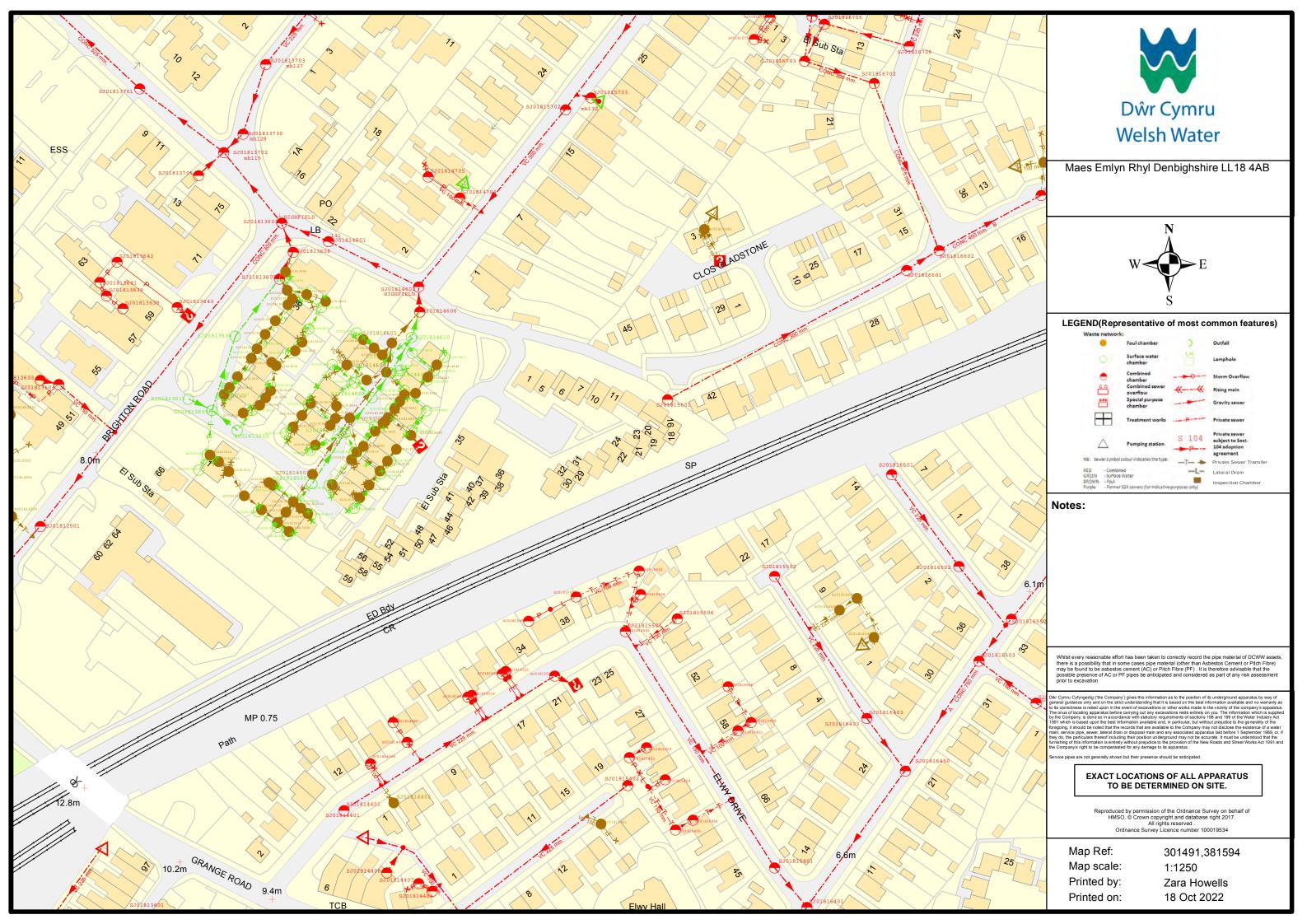
WS06

Projec	t Name	: Maes Emly	yn		Client: [	Denbighshi	re CC			Date: 19/0	1/2023		
Locati	on: Rhy	I			Contrac	tor: Region	nal Drillin	g		Co-ords: E	301491.00	N381621.0	00
Projec	t No. : 2	2841			Crew N	ame: Regio	onal Dillir	ng		Drilling Eq	uipment: Pı	emier Com	pact 110
Bor	ehole N WS06			Type LS	6	Level 50m AoD		Logged	Ву	l.	cale		Number
	Water			ເຣ າ Situ Testir		Depth	Level	AB			:35		et 1 of 1
Well	Strikes	Depth (m)		Result		(m)	(m)	Legend		Strat	um Descrip	tion	
		0.20	ES			0.40	6.10		SILT wit (TOPSC	th rootlets. DIL)	lightly gravell ense brown g s)		ndy
		1.20	SPT	N=3 (0,1/0,	1,0,2)				No samp	ole recover	y due to wa	ter	1 -
		2.00	SPT	N=30 (5,5/6	,8,8,8)	2.00	4.50		Still gre	y sandy CLA	Y.		2
		2.30	D							AL TILĹ) s brown in	<u>colo</u> ur		-
		3.00	SPT	N=26 (5,5/5	5,7,7,7)								3 -
		4.00 4.00	D SPT	N=19 (3,4/4	-,4,5,6)				No samp	ole recover	y due to wa	ter	4 -
		5.00	SPT	N=17 (3,3/4	.,4,4,5)	5,45	1.05			End of	Borehole at 5	.450m	5 -
													6 -
Depth I	Hole Diame		Casing (	Diameter Diameter	Depth To	p Depth Ba	Chiselling ase Dura	ation	Tool	Depth Top	Inclination Depth Base	and Orientation Inclination	Orientation
1.00 2.00 3.00 4.00 5.48	) ) ) ) 5	200 100 92 79 70	1.00 2.00	150 150	opai 10	- Popul De	- Dulie		.551	23541100			CGIRGHOTT



## Appendix E DCWW Sewer Plans & Correspondence





## **Adam McCulloch**

Jake MacMillan < Jake. MacMillan@dwrcymru.com> From:

24 August 2023 10:49 Sent: Adam McCulloch To: RE: PPA0007341 Subject:

Hi Adam,

Apologies for the delayed response and thank you for your patience. In this instance I appreciate that investigations have taken place albeit the historic surveys lack the detail we would expect. We would therefore be amenable to an attenuated rate here, however we generally look to achieve a lower rate of 5 l/s, I would therefore request that you endeavour to achieve an attenuated rate in line with these flows.

## Best regards,



#### Jake MacMillan

Development Planning Officer | Developer Services Dŵr Cymru Welsh Water



T: 0800 917 2652 | E: 45xxx | M: 07557860559

A: PO Box 3146, Cardiff, CF30 0EH



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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Sent: Wednesday, August 16, 2023 2:05 PM

To: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good afternoon Jake,

Regarding my previous email dated 13<sup>th</sup> of July, please could you advise when we may receive a response.

Kind Regards,

### Adam McCulloch

**Environmental Consultant** 



adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Sent: Friday, July 7, 2023 4:35 PM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Apologies for the delayed response and thank you for providing this report.

Can you clarify further on how the audio tests confirm connectivity to the sewer, and can you forward any reports you have on this?

We would usually expect a detailed cctv report to confirm connectivity, can you advise if it is possible to achieve

As the proposal is due for a SAB application, I'm unable to provide an agreement in principle, as this is the premise for the application/consultations.

### Best regards,

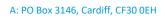


#### Jake MacMillan

Development Planning Officer | Developer Services Dŵr Cymru Welsh Water



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E: developer.services@dwrcymru.com

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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Sent: Thursday, June 15, 2023 9:28 AM

To: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good morning Jake,

Please see attached a Drainage Layout from Invek surveys regarding the proposed development at Maes Emlyn, Rhyl. The survey shows that all surface water currently drains to the existing DCWW public combined sewer in the site's north-eastern extent.

Please can you advise if a surface water connection to the public combined sewer is acceptable.

Kind Regards,

#### Adam McCulloch

**Environmental Consultant** 

adam.mcculloch@waterco.co.uk

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From: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Sent: Thursday, May 11, 2023 3:07 PM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

This is preferred in order to get a fuller understanding of the historical drainage arrangement, as a GPR isn't conclusive. We can arrange a sewer trace if you'd prefer however would be developer funded.

#### Best regards,



### Jake MacMillan

Development Planning Officer | Developer Services Dŵr Cymru Welsh Water



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A: PO Box 3146, Cardiff, CF30 0EH



W: dwrcymru.com



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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

**Sent:** 11 May 2023 10:32

**To:** Jake MacMillan < <u>Jake.MacMillan@dwrcymru.com</u>>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good morning Jake,

No further intrusive CCTV drainage investigations have been undertaken. Please could you advise if this is required.

Kind Regards,

## Adam McCulloch

**Environmental Consultant** 



adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < <u>Jake.MacMillan@dwrcymru.com</u>>

Sent: Tuesday, May 9, 2023 9:16 AM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for providing this report which shows the intrusive soakaway testing results. Can you also advise if there has been any intrusive/cctv investigations looking at the historical drainage arrangement, building further on the GPR survey?

#### Best regards,



#### Jake MacMillan

Development Planning Officer | Developer Services Dŵr Cymru Welsh Water



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A: PO Box 3146, Cardiff, CF30 0EH



W: dwrcymru.com



E: developer.services@dwrcymru.com

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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Sent: 05 May 2023 16:34

To: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good afternoon Jake,

Intrusive ground investigations have been undertaken by GroundSolve Ltd. The intrusive ground investigation comprised 6No. windowless sample (WS) boreholes and Infiltration tests. Please see attached for your reference.

Kind Regards,

## Adam McCulloch

**Environmental Consultant** 



adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < <u>Jake.MacMillan@dwrcymru.com</u>>

Sent: Thursday, April 27, 2023 10:16 AM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for providing the below, can you advise if there have been any cctv surveys or intrusive investigations that have taken place? Can you also advise what stage the site is at currently?

#### Best regards,



#### Jake MacMillan

Development Planning Officer | Developer Services Dŵr Cymru Welsh Water



T: 0800 917 2652 | E: 45xxx | M: 07557860559

A: PO Box 3146, Cardiff, CF30 0EH



W: dwrcymru.com

E: developer.services@dwrcymru.com

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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Sent: 25 April 2023 16:19

To: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good afternoon Jake,

A GPR survey has been undertaken by PM Surveys UK Ltd which shows the existing connection to the public combined manhole in the eastern extent of the site. Please see attached for your reference.

Kind Regards,

## Adam McCulloch

**Environmental Consultant** 



📽 adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

**Sent:** Tuesday, April 25, 2023 2:53 PM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for the below email, can you confirm what investigations have been done to confirm that there is an existing connection to the public combined sewer?

Best regards,

#### Jake MacMillan

Development Planning Officer | Developer Services



T: 0800 917 2652 | E: 45xxx | M: 07557860559 A: PO Box 3146, Cardiff, CF30 0EH



If we've gone the extra mile to provide you with excellent service, let us know. You can nominate an individual or team for a Diolch award through our website

From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

**Sent:** 24 April 2023 09:44

**To:** Jake MacMillan < <u>Jake.MacMillan@dwrcymru.com</u>>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Hi Jake,

Further to your below email relating to PPA0007341, we have obtained a ground investigation report (see attached) from GroundSolve undertaken in January 2023.

The intrusive ground investigation shows that groundwater was encountered within all of the exploratory hole locations within either the Made Ground or wind-blown sand, at depths between 0.60 metres below ground level (m.bgl) and 1.00m.bgl. Infiltration tests were undertaken as part of the intrusive ground investigation and show that all of the trial pits failed the first cycle, as the water failed to percolate through the underlying superficial deposits.

As infiltration is not suitable, a connection to a watercourse is the next consideration. The nearest watercourse is The Cut which is located approximately 260m east of the site. The site is separated from The Cut by third party, urbanised land including a railways line. A connection to a watercourse is therefore not a feasible option.

There are no surface water sewers in the vicinity of the site (which do not connect into the public combined sewer system). As a result of these findings, we propose to connect to the public combined sewer located within the site's eastern extent per the existing scenario. I attach a GPR survey which demonstrates the existing connection. The 'foul drain' identified on the GPR accommodates rainfall runoff from the roof and is a combined drain.

We propose a surface water discharge rate of 13.7 l/s which provides 30% betterment on the existing 1 in 1 year brownfield runoff rate. Lower discharge rates have been considered however increase the amount and depth of attenuation storage required, which in turn limits the feasibility of a gravity connection.

Please can you advise if a surface water connection to the public combined sewer at 13.7 l/s is acceptable. Please don't hesitate to contact me if you have any questions.

Kind Regards,

### Adam McCulloch

**Environmental Consultant** 

adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

Sent: Thursday, November 17, 2022 10:41 AM

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for the email, unfortunately we aren't a position to agree in principle, until the hierarchy has been exhausted and a historical connection has been confirmed.

## Best regards,



#### Jake MacMillan

Development Control Officer | Developer Services Dŵr Cymru Welsh Water



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A: PO Box 3146, Cardiff, CF30 0EH



E: developer.services@dwrcymru.com

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From: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Sent: 17 November 2022 09:26

To: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Hi Jake,

To confirm, the number of proposed units will be 35No. (including flats).

Regarding drainage on site we are awaiting results of ground investigations, however we assume that discharge is currently made to the public combined sewer.

Could a connection point to the sewer be agreed in principle for surface water at the stage, subject to review of infiltration techniques?

Kind Regards,

## **Adam McCulloch**

**Environmental Consultant** 

adam.mcculloch@waterco.co.uk Teams:

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Please consider the environment before printing this email.

From: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

**Sent:** 16 November 2022 10:34

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for the reply, so just to confirm this proposal comprises a demolition of 59 units and replacement with 56 units (35 dwellings and 21 flats).

In regards to the surface water I can see you've proposed to discharge to the public sewer, I am happy to continue these discussions via email. With the nature of this development a SAB application will be required which will assess the feasibility of sustainable drainage (collection, infiltration, surface water body, highways drainage/surface water sewer). Can you advise where the existing development discharges to and what investigations have been made into the sustainable drainage hierarchy? I'm also happy to discuss this at a later date if you're not a position to discuss at this stage.

#### Best regards,



#### Jake MacMillan

Development Control Officer | Developer Services Dŵr Cymru Welsh Water



T: 0800 917 2652 | E: 45xxx | M: 07557860559

A: PO Box 3146, Cardiff, CF30 0EH



W: dwrcymru.com

E: developer.services@dwrcymru.com

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From: Adam McCulloch <a href="mailto:adam.mcculloch@waterco.co.uk">adam.mcculloch@waterco.co.uk</a>

**Sent:** 15 November 2022 10:18

To: Jake MacMillan < Jake. MacMillan@dwrcymru.com>

Subject: RE: PPA0007341

\*\*\*\*\*\* External Mail \*\*\*\*\*\*

Good morning Jake,

Further to your previous email, please see attached an existing site layout plan. The existing layout comprises 59 no. dwellings in the form of houses and apartments. All existing properties are to be demolished.

The development is currently connected to the public sewer system.

If you require any further information, please don't hesitate to contact me.

Kind Regards,

### Adam McCulloch

**Environmental Consultant** 

Teams: adam.mcculloch@waterco.co.uk

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Please consider the environment before printing this email.

From: Jake MacMillan < Jake.MacMillan@dwrcymru.com>

Sent: 14 November 2022 12:11

To: Adam McCulloch <adam.mcculloch@waterco.co.uk>

Subject: RE: PPA0007341

Hi Adam,

Thank you for submitting the above pre planning application. It appears that this site comprises a redevelopment, can you advise on the existing housing structure that will be demolished? Can you also advise if this is connected to the public sewerage network?

## Best regards,



#### Jake MacMillan

Development Control Officer | Developer Services Dŵr Cymru Welsh Water



T: 0800 917 2652 | E: 45xxx | M: 07557860559

A: PO Box 3146, Cardiff, CF30 0EH



W: dwrcymru.com



E: developer.services@dwrcymru.com

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From: formsubmission@dwrcymru.com <formsubmission@dwrcymru.com>

Sent: 09 November 2022 16:35

To: Services Developer <developer.services@dwrcymru.com>

Cc: DSRevenue < DSRevenue@dwrcymru.com>

Subject: PPA0007341

### **Applicant details**

Company Name: Waterco

Title: Mr

First Name: Adam

Surname: McCulloch

House Name:
House Number:
Street 1:
Street 2:
Town: Ruthin
County: Denbighshire
Post code: LL15 1NJ
Contact Telephone Number (1): 01824702220
Contact Telephone Number (2):
Email address: adam.mcculloch@waterco.co.uk
Name and Address of Agent
Company Name:
Title:
First Name:
Surname:
House Name:
House Number:
Street 1:
Street 2:
Town:
County:
Post code:
Contact Telephone Number (1):
Contact Telephone Number (2):
Email address:
Development Details

What is the nature of your proposal: Residential re-development

Name of Development: Maes Emlyn

House Name:
House Number:
Plots:
Street 1:
Street 2:
Town: Rhyl
County: Denbighshire
Post Code: LL18 4AB
National Grid Reference of development eg ST123456: SJ014816
X Coordinates: 301491
Y Coordinates: 381610
Quantity of Water
Important Consumption and flow rates are essential for the correct sizing of the supply and meter:
Please provide your estimated water quantities in litres for all nondomestic connections:
Minimum flow:
Maximum flow:
Quantity per 24 hours:
We need them to determine whether the existing distribution network can accommodate this demand without reinforcement ie laying new mains and these will form the basis of the supply agreement between you and us:
Development SupplyDemand
Is there an existing DCWW supply on site: Yes
Domestic Development
Please give details of the proposed development:
Housing total properties:
Units: 35
Area ha: 1
House Type:
Terraced Units: 6

13 bedrooms Units: 14
4 bedrooms Units:
Flats:
Flats Number of storeys: 3
Flats Units: 21
Flat 1 bedroom Units: 18
Flat 2 bedroom: 3
Flat 2 bedroom: 0
Commercial Development
What is the intended use of the premises:
IndustrialWarehousing:
Staff: 0
Units:
Area ha:
Hotel:
Number of rooms:
Leisure:
Staff:
Units:
Area ha:
Retail fooddrink:
Staff:
Units:
Area ha:
Retail non fooddrink:
Staff:
Units:

Semi detachedDetached Units: 8

Area ha:
Office:
Staff:
Units:
Area ha:
Other:
Staff:
Units:
Area ha:
If Other please specify:
Drainage Details
Method of surface water disposal: Public Sewer
Surface Water Attenuation: Yes
Is it proposed to use SuDS features within the site if so which type: Attenuation Tank
Method of foul water disposal: Public Sewer
Method of highway drainage disposal: Public Sewer
Will the proposal require a newamended trade effluent consent: No
Use of CommercialIndustrial Premises:
Specify trade andor type of commercial or industrial activity eg Manufacturing processing mechanical electrical etc Please provide as much detail as possible:
Will the development require the use of potable water in the treatmentcleaning process: No
Proposed Development Programme
Phasing of development:
Year: 2023
Type of Development: Residential
Units: 35
Area ha: 1
Year:

Type of Development:
Units:
Area ha:
Year:
Type of Development:
Units:
Area ha:
Year:
Type of Development:
Units:
Area ha:
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Type of Development:
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Type of Development:
Units:
Area ha:
Year:
Type of Development:
Units:
Area ha:

**Planning Status** 

Is the site allocated in a Development Plan: No
Is Yes please provide Development Plan Name:
Development Plan Site Reference:
If Brownfield site what iswas the previous site use eg school factory etc: Residential
Historical Information
If the site is a Brownfield site please provide information regarding the following:
Water Consumption:
Units:
Area ha:
Foul Sewage discharge rate:
Units:
Area ha:
Surface Water discharge rate:
Units:
Area ha:
Id: 071e2e41-bc90-4278-8628-dcc4dfe4305a
Price: 85.20
File Uploads
Do you have a Location Plan to upload: Yes
Please select the Location Plan file: /PPA/ea386961-11b3-4e9d-a75d-1508ce876746/LocationPlan.pdf
Please be aware that it is not possible to upload files when using an iPad or iPhone:
Dwr Cymru Welsh Water is firmly committed to water conservation and promoting water efficiency. Please log on to our website www.dwrcymru.com/waterefficiency to find out how you can become water wise. Mae Dwr Cymru Welsh Water wedi ymrwymo i warchod adnoddau dwr a hyrwyddo defnydd dwr effeithiol. Mae cyngor i' ch helpu i ddefnyddio dwr yn ddoeth yn www.dwrcymru.com/waterefficiency  ***********************************
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Dwr Cymru Welsh Water is firmly committed to water conservation and promoting water efficiency. Please log on to our website <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a> to find out how you can become water wise. Mae Dwr Cymru Welsh Water wedi ymrwymo i warchod adnoddau dwr a hyrwyddo defnydd dwr effeithiol. Mae cyngor i' ch helpu i ddefnyddio dwr yn ddoeth yn <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a>
**************************************
Dwr Cymru Welsh Water is firmly committed to water conservation and promoting water efficiency. Please log on to our website <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a> to find out how you can become water wise. Mae Dwr Cymru Welsh Water wedi ymrwymo i warchod adnoddau dwr a hyrwyddo defnydd dwr effeithiol. Mae cyngor i' ch helpu i ddefnyddio dwr yn ddoeth yn <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a>
**************************************
Dwr Cymru Welsh Water is firmly committed to water conservation and promoting water efficiency. Please log on to our website <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a> to find out how you can become water wise. Mae Dwr Cymru Welsh Water wedi ymrwymo i warchod adnoddau dwr a hyrwyddo defnydd dwr effeithiol. Mae cyngor i' ch helpu i ddefnyddio dwr yn ddoeth yn <a href="https://www.dwrcymru.com/waterefficiency">www.dwrcymru.com/waterefficiency</a>
**************************************
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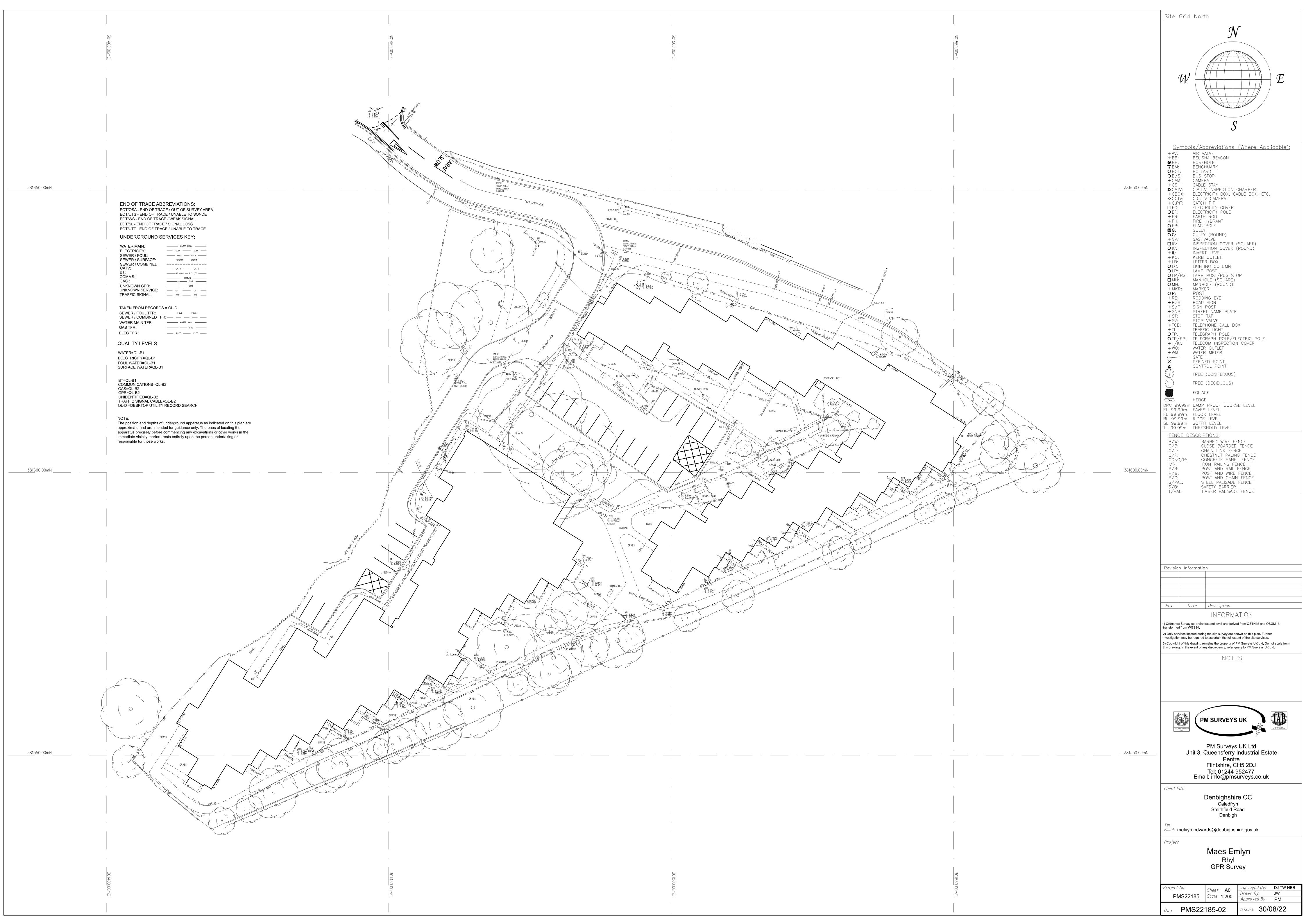
OLT. Mae'r neges e-bost yma ac unrhyw ffeil sydd ynghlwm wrthi'n gyfrinachol. Os nad chi yw'r derbynnydd a enwir neu os ydych chi'n credu eich bod wedi derbyn y neges yma ar gam, dylech ei dileu o'ch system ar unwaith a hysbysu'r anfonwr. Cwmni sydd wedi ei gofrestru yng Nghymru yw Dŵr Cymru Cyf (yn masnachu fel Dŵr Cymru), ei rif cofrestredig yw 02366777, ,, ac mae ei swyddfa gofrestredig yn Linea, Heol Fortran, Llaneirwg, Caerdydd, CF3 OLTHERSTERMENT CAERDY
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dwr yn ddoeth yn www.dwrcymru.com/waterefficiency

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## Appendix F GPR Survey





## **Appendix G** Invek Surveys Drainage Layout





## Appendix H Proposed Development Plan





## Appendix I SAB Consultation Response



## **Adam McCulloch**

From: Daniel Jones < Daniel.Jones@denbighshire.gov.uk>

Sent: 10 January 2023 14:33
To: Adam McCulloch

**Cc:** Land Drainage Consultations

**Subject:** RE: 14973-SAB Pre-Application-Maes Emlyn [Filed 10 Jan 2023 14:53]

Categories: Filed by Mail Manager

Good Afternoon Adam,

Thank you for the information below regarding proposals at Maes Emlyn, Rhyl.

We would have no objections in principle to your intention of discharging the surface water from the site into the public combined sewer, providing the following conditions are met:

- 1. Evidence that the hierarchy has been followed and that you have explored and exhausted alternative surface water drainage options.
- 2. Confirmation from Welsh Water that they are happy to accepts flows from the site.

Kind regards,

### Daniel Jones BSc (Hons)

Swyddog Perygl Llifogydd / Flood Risk Officer

Cyngor Sir Ddinbych / Denbighshire County Council

Priffyrdd a Gwasanaethau Amgylcheddol / Highways & Environmental Services

Ffon/Phone: 01824 706822 / 07824 409601

Gwefan/Website: www.sirddinbych.gov.uk / www.denbighshire.gov.uk

From: Adam McCulloch [mailto:adam.mcculloch@waterco.co.uk]

Sent: 23 December 2022 13:16

To: Land Drainage Consultations < landdrainage.consultations@denbighshire.gov.uk>

Subject: RE: 14973-SAB Pre-Application-Maes Emlyn

Proposed residential development at Maes Emlyn, Rhyl, Denbighshire, LL18 4AB. Grid Reference: 301448, 381587.

Dear Sir/Madam,

We are currently undertaking a Drainage Strategy at the above address. The proposed development is for the demolition of 59No. existing residential dwellings and erection of 38No. residential dwellings with associated access roads and parking. Please find attached the completed SuDS pre-application form and supporting documentation.

If you require any further information to process my request, please do not hesitate to contact me.

Kind Regards,

#### Adam McCulloch

**Environmental Consultant** 

Teams: adam.mcculloch@waterco.co.uk

We're recruiting! For more information, please take a look at our website.

For email confidentiality, limitations and company details please see our disclaimer webpage. Registered in Wales under company 3577754. Waterco Ltd, Eden Court, Ruthin LL15 1NJ. Please click for our GDPR policy.



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http://www.denbighshire.gov.uk

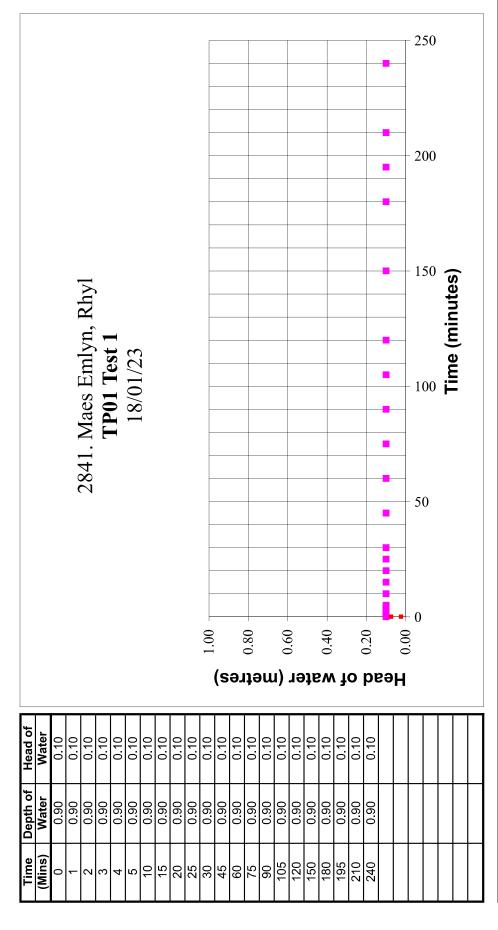
Mae'r wybodaeth a gynhwysir yn yr e-bost hwn ac unrhyw ffeiliau a drosglwyddir gydag o wedi eu bwriadu yn unig ar gyfer pwy bynnag y cyfeirir ef ato neu atynt. Os ydych wedi derbyn yr e-bost hwn drwy gamgymeriad, hysbyswch yr anfonwr ar unwaith os gwelwch yn dda. Mae cynnwys yr e-bost yn cynrychioli barn yr unigolyn(ion) a enwir uchod ac nid yw o angenrheidrwydd yn cynrychioli barn Cyngor Sir Ddinbych. Serch hynny, fel Corff Cyhoeddus, efallai y bydd angen i Gyngor Sir Ddinbych ddatgelu'r e-bost hwn [neu unrhyw ymateb iddo] dan ddarpariaethau deddfwriaethol.

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## Appendix J Soakaway Test Results



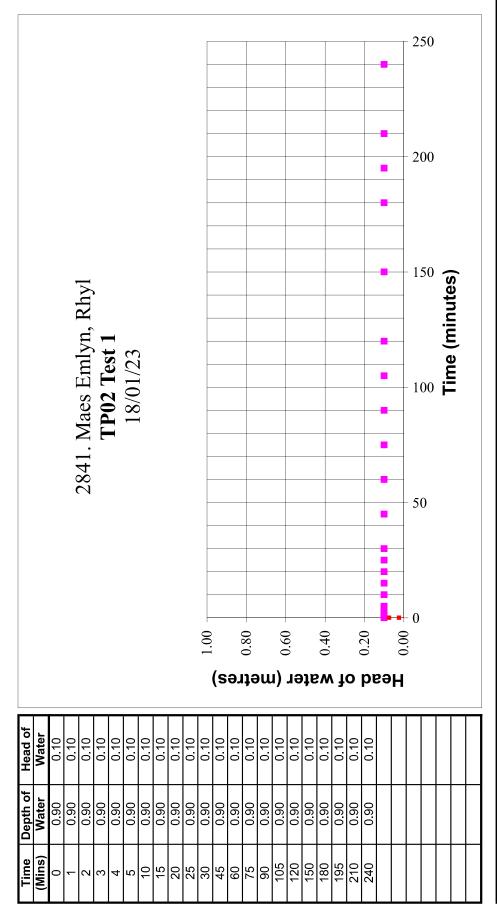




Depth	Length	Width	Head at 75% Vol	Head at 25% Vol	Water depth(t=0)	Effective Area	Vp75-Vp25	Adjustment Factor *	t75	t25	Infiltration
(m)	(m)	(m)	(m)	(m)	(m)	Ap50 m2	Volume of water m3		(mins)	(mins)	rate m/s
1.00	1.70	0.70	0.075	0.03	0.10	1.430	090'0	1.00		0	N/A

\* with gravel filter adjustment factor = 0.30, with no filter = 1.0 Note: The test failed to reach the 25% & 75% head

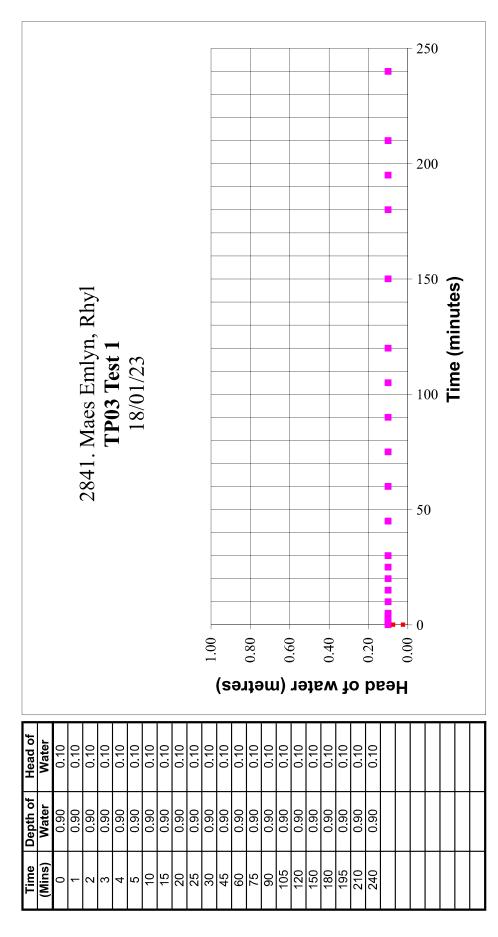




Depth	Length	Width	Head at 75% Vol	Head at 25% Vol	Water depth(t=0)	Effective Area	Vp75-Vp25	Adjustment Factor *	t75	t25	Infiltration
(m)	(m)	(w)	(m)	(m)	(m)	Ap50 m2	Volume of water m3		(mins)	(mins)	rate m/s
1.00	1.65	02'0	0.075	0.03	0.10	1.390	0.058	1.00			N/A

\* with gravel filter adjustment factor = 0.30, with no filter = 1.0 Note: The test failed to reach the 25% & 75% head





Depth	Length	Width	Head at 75% Vol	Head at 25% Vol	Water depth(t=0)	Effective Area	Vp75-Vp25	Adjustment Factor *	t75	t25	Infiltration
(m)	(m)	(m)	(m)	(m)	(m)	Ap50 m2	Volume of water m3		(mins)	(mins)	rate m/s
1.00	1.60	0.70	0.075	0.03	0.10	1.350	950'0	1.00			N/A

\* with gravel filter adjustment factor = 0.30, with no filter = 1.0 Note: The test failed to reach the 25% & 75% head

## Appendix K ReFH2 Greenfield Runoff Rates



DOCUMENT VERIFICATION RECORD				
Project:	14973 – Maes Emlyn, Rhyl			
Client:	TACP Architects Ltd			
Report Title:	14973-Drainage Strategy-01			
Date:	01/02/23			

DOCUMENT REVIEW & APPROVAL				
Author:	Adam McCulloch			
Checker: Aled Williams BSc (Hons) MCIWEM C.WEM				
Approver:	Nigel Jones BEng (Hons) CEng MICE			

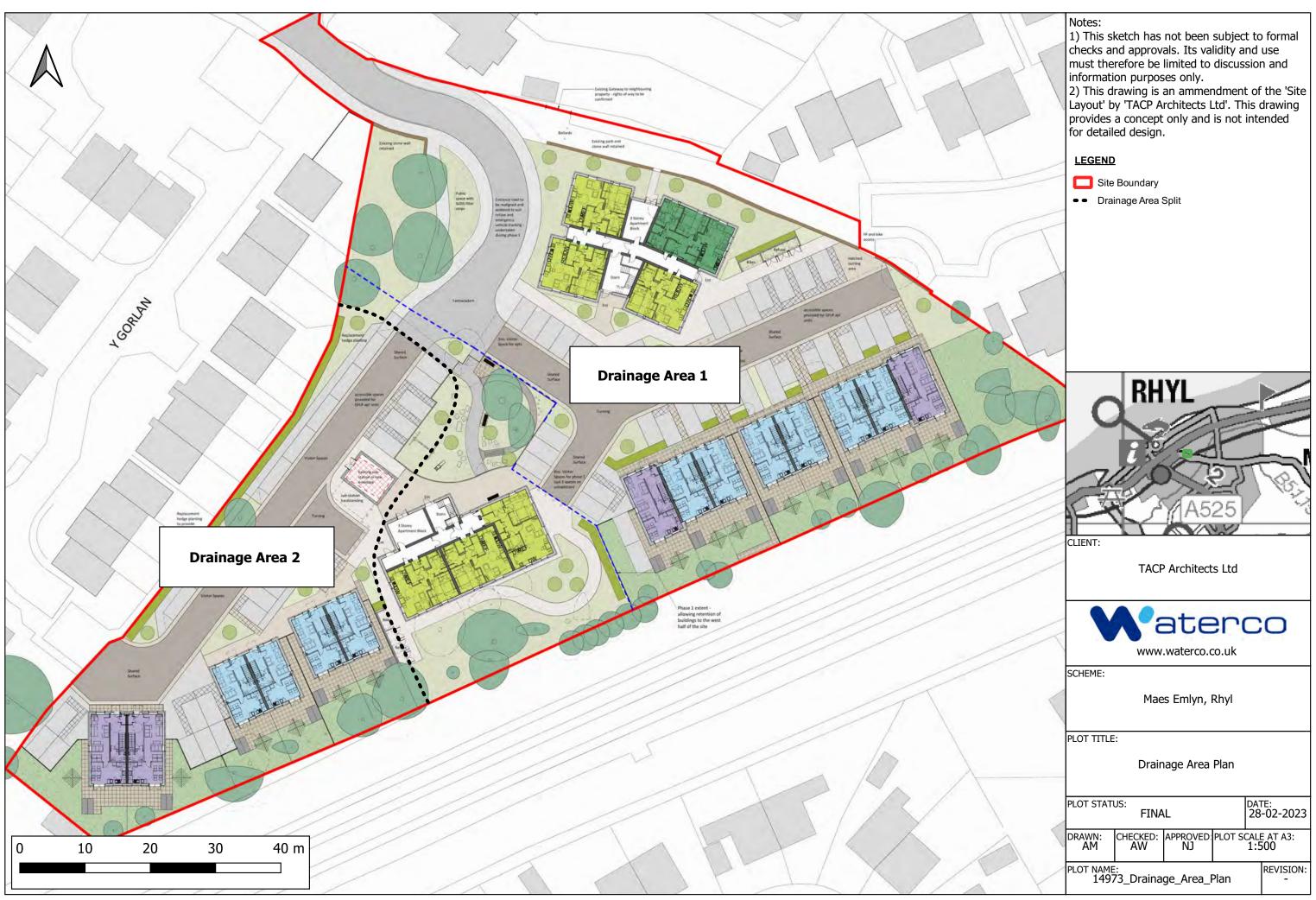
ReFH2 RUNOFF RATES*					
Return Period (Years)	As-rural Peak Flow (I/s)				
1	0.58				
2	0.67				
5	1.00				
10	1.27				
30	1.84				
50	2.19				
75	2.51				
100	2.76				
200	3.43				
1000	5.21				

<sup>\*</sup>Runoff Rates printed from the ReFH Flood Modelling software package



# Appendix L Drainage Areas Plan





# **Appendix M** MicroDrainage Attenuation Storage Estimates



Waterco Ltd		Page 1
Eden Court	Maes Emlyn	
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainago
File	Checked by AW	pran racie
XP Solutions	Source Control 2020.1.3	

Storm Event		Max	Max	Max	Max	Status	
	Even	C	(m)	(m)	Control (1/s)	(m³)	
15	min	Summer	9.444	0.444	3.6	135.8	ОК
30	min	Summer	9.585	0.585	3.6	179.0	O K
60	min	Summer	9.734	0.734	3.6	224.6	Flood Risk
120	min	Summer	9.867	0.867	3.6	265.2	Flood Risk
180	min	Summer	9.932	0.932	3.6	285.2	Flood Risk
240	min	Summer	9.967	0.967	3.6	296.0	Flood Risk
360	min	Summer	9.991	0.991	3.6	303.3	Flood Risk
480	min	Summer	9.991	0.991	3.6	303.3	Flood Risk
600	min	Summer	9.977	0.977	3.6	299.1	Flood Risk
720	min	Summer	9.958	0.958	3.6	293.1	Flood Risk
960	min	Summer	9.918	0.918	3.6	280.9	Flood Risk
1440	min	Summer	9.840	0.840	3.6	257.0	Flood Risk
2160	min	Summer	9.731	0.731	3.6	223.6	Flood Risk
2880	min	Summer	9.627	0.627	3.6	191.8	ОК
4320	min	Summer	9.434	0.434	3.6	132.8	ОК
5760	min	Summer	9.308	0.308	3.6	94.2	ОК
7200	min	Summer	9.230	0.230	3.6	70.4	ОК
8640	min	Summer	9.181	0.181	3.5	55.3	ОК
10080	min	Summer	9.149	0.149	3.3	45.5	O K

	Stor	m	Rain	${\tt Flooded}$	Discharge	Time-Peak
	Even	t	(mm/hr)	Volume	Volume	(mins)
				(m³)	(m³)	
			156.101	0.0	136.6	16
30	min	Summer	103.795	0.0	181.1	31
60	min	Summer	66.103	0.0	233.9	62
120	min	Summer	40.250	0.0	284.7	122
180	min	Summer	29.726	0.0	315.2	182
240	min	Summer	23.809	0.0	336.5	242
360	min	Summer	17.216	0.0	364.7	360
480	min	Summer	13.656	0.0	385.5	480
600	min	Summer	11.385	0.0	401.5	600
720	min	Summer	9.798	0.0	414.4	656
960	min	Summer	7.701	0.0	433.6	768
1440	min	Summer	5.446	0.0	457.8	1024
2160	min	Summer	3.819	0.0	486.6	1448
2880	min	Summer	2.961	0.0	503.0	1848
4320	min	Summer	2.058	0.0	524.4	2552
5760	min	Summer	1.598	0.0	542.8	3240
7200	min	Summer	1.330	0.0	564.8	3960
8640	min	Summer	1.155	0.0	588.6	4592
10080	min	Summer	1.034	0.0	614.8	5256

Waterco Ltd		Page 2
Eden Court	Maes Emlyn	
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainago
File	Checked by AW	nigii iade
XP Solutions	Source Control 2020.1.3	<u>'</u>

Storm Event		Max Level	Max Depth	Max Control	Max Volume	Status	
			(m)	(m)	(l/s)	(m³)	
15	min	Winter	9.444	0.444	3.6	135.7	ОК
30	min	Winter	9.585	0.585	3.6	179.0	O K
60	min	Winter	9.734	0.734	3.6	224.5	Flood Risk
120	min	Winter	9.867	0.867	3.6	265.4	Flood Risk
180	min	Winter	9.934	0.934	3.6	285.7	Flood Risk
240	min	Winter	9.970	0.970	3.6	296.7	Flood Risk
360	min	Winter	9.995	0.995	3.6	304.6	Flood Risk
480	min	Winter	9.998	0.998	3.6	305.3	Flood Risk
600	min	Winter	9.986	0.986	3.6	301.9	Flood Risk
720	min	Winter	9.967	0.967	3.6	295.9	Flood Risk
960	min	Winter	9.917	0.917	3.6	280.7	Flood Risk
1440	min	Winter	9.824	0.824	3.6	252.2	Flood Risk
2160	min	Winter	9.678	0.678	3.6	207.3	O K
2880	min	Winter	9.516	0.516	3.6	157.8	O K
4320	min	Winter	9.289	0.289	3.6	88.4	O K
5760	min	Winter	9.169	0.169	3.4	51.6	O K
7200	min	Winter	9.114	0.114	3.1	34.8	O K
8640	min	Winter	9.095	0.095	2.8	29.2	O K
10080	min	Winter	9.085	0.085	2.5	26.0	ОК

	Stor Even		Rain (mm/hr)		Discharge Volume (m³)	Time-Peak (mins)
15	min	Winter	156.101	0.0	136.6	16
30	min	Winter	103.795	0.0	181.1	31
60	min	Winter	66.103	0.0	233.9	62
120	min	Winter	40.250	0.0	284.7	120
180	min	Winter	29.726	0.0	315.2	178
240	min	Winter	23.809	0.0	336.5	236
360	min	Winter	17.216	0.0	364.7	352
480	min	Winter	13.656	0.0	385.5	464
600	min	Winter	11.385	0.0	401.5	576
720	min	Winter	9.798	0.0	414.4	678
960	min	Winter	7.701	0.0	433.6	788
1440	min	Winter	5.446	0.0	458.0	1082
2160	min	Winter	3.819	0.0	486.6	1556
2880	min	Winter	2.961	0.0	503.0	1932
4320	min	Winter	2.058	0.0	524.5	2632
5760	min	Winter	1.598	0.0	542.8	3232
7200	min	Winter	1.330	0.0	564.8	3824
8640	min	Winter	1.155	0.0	588.6	4496
10080	min	Winter	1.034	0.0	614.8	5152

Waterco Ltd		Page 3
Eden Court	Maes Emlyn	-
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainage
File	Checked by AW	pianiacje
XP Solutions	Source Control 2020.1.3	<del>'</del>

## Rainfall Details

Rainfall Model						FEH
Return Period (years)						100
FEH Rainfall Version						2013
Site Location	GB	301481	381595	SJ	01481	81595
Data Type						Point
Summer Storms						Yes
Winter Storms						Yes
Cv (Summer)						1.000
Cv (Winter)						1.000
Shortest Storm (mins)						15
Longest Storm (mins)						10080
Climate Change %						+40

#### Time Area Diagram

Total Area (ha) 0.354

Time (mins) Area
From: To: (ha)

0 1 0.354

Waterco Ltd		Page 4
Eden Court	Maes Emlyn	
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainage
File	Checked by AW	Diamage
XP Solutions	Source Control 2020.1.3	•

#### Model Details

Storage is Online Cover Level (m) 10.000

#### Tank or Pond Structure

Invert Level (m) 9.000

# Depth (m) Area $(m^2)$ Depth (m) Area $(m^2)$

0.000 306.0 1.000 306.0

#### Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0090-3600-1000-3600
Design Head (m)	1.000
Design Flow (1/s)	3.6
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	90
<pre>Invert Level (m)</pre>	8.995
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

# Control Points Head (m) Flow (1/s) Design Point (Calculated) 1.000 3.6 Flush-Flom 0.300 3.6 Kick-Flo® 0.631 2.9

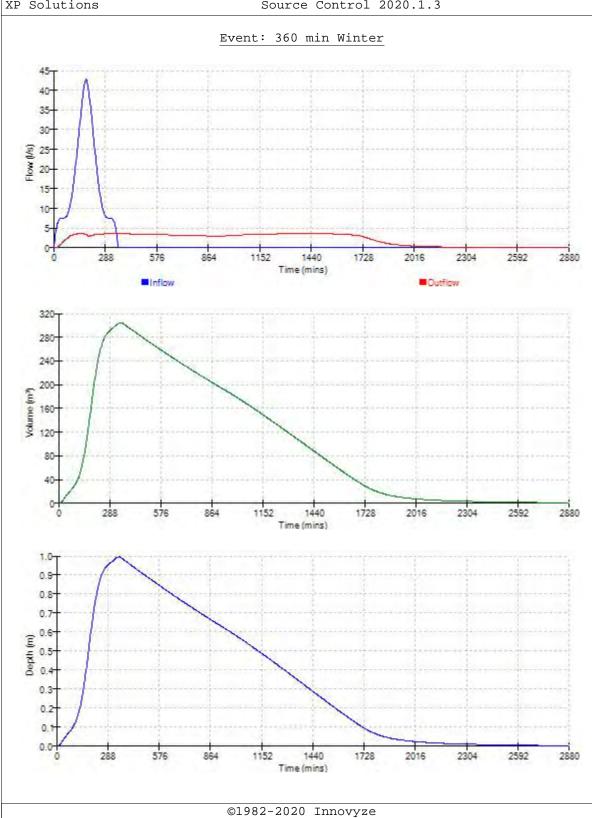
3.1

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

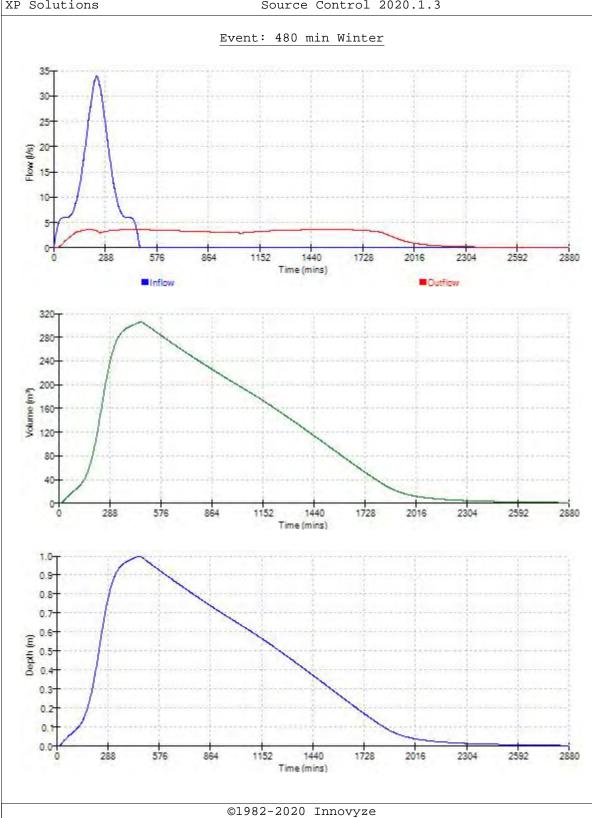
Mean Flow over Head Range

Depth (m)	Flow (1/s)	Depth (m)	Flow (1/s)	Depth (m)	Flow (1/s)
0.100	2.8	1.600	4.5	5.000	7.6
0.200	3.5 3.6	1.800	4.7 5.0	5.500	8.0
0.400	3.5	2.200	5.2	6.500	8.6
0.500	3.4	2.400	5.4	7.000	9.0
0.600	3.1	2.600	5.6	7.500	9.3
0.800	3.2	3.000	6.0	8.000	9.5
1.000	3.6	3.500	6.5	8.500	9.8
1.200	3.9	4.000	6.9	9.000	10.1
1.400	4.2	4.500	7.3	9.500	10.4

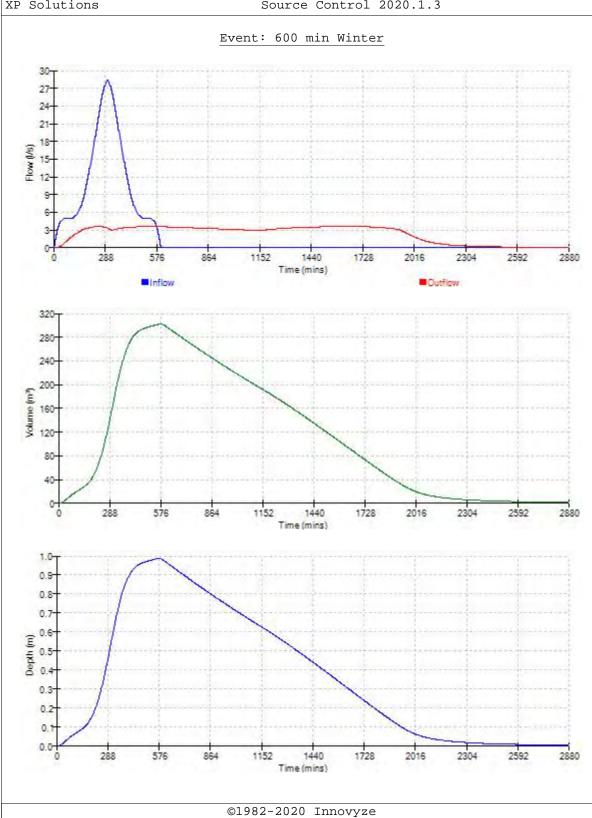
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Eden Court	Maes Emlyn	E
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainage
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Waterco Ltd		Page 6
Eden Court	Maes Emlyn	E
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainage
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YD Solutions	Source Control 2020 1 3	<u> </u>



Waterco Ltd		Page 7
Eden Court	Maes Emlyn	
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 1	Micro
Date 30/08/2023	Designed by AM	Drainage
File	Checked by AW	nian larie
XP Solutions	Source Control 2020.1.3	



Waterco Ltd		Page 1
Eden Court	Maes Emlyn	
Lon Parcwr Business Park	Rhyl	
Denbighshire LL15 1NJ	Drainage Area 2	Micro
Date 30/08/2023	Designed by AM	Designation
File	Checked by AW	niamarie
XP Solutions	Source Control 2020.1.3	'

	Stor Even		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status
15	min	Summer	9.441	0.441	1.3	53.4	ОК
30	min	Summer	9.582	0.582	1.3	70.4	ОК
60	min	Summer	9.729	0.729	1.3	88.2	Flood Risk
120	min	Summer	9.861	0.861	1.3	104.2	Flood Risk
180	min	Summer	9.927	0.927	1.4	112.2	Flood Risk
240	min	Summer	9.962	0.962	1.4	116.5	Flood Risk
360	min	Summer	9.987	0.987	1.4	119.5	Flood Risk
480	min	Summer	9.989	0.989	1.4	119.6	Flood Risk
600	min	Summer	9.976	0.976	1.4	118.1	Flood Risk
720	min	Summer	9.957	0.957	1.4	115.8	Flood Risk
960	min	Summer	9.919	0.919	1.4	111.2	Flood Risk
1440	min	Summer	9.844	0.844	1.3	102.1	Flood Risk
2160	min	Summer	9.743	0.743	1.3	89.9	Flood Risk
2880	min	Summer	9.657	0.657	1.3	79.4	O K
4320	min	Summer	9.495	0.495	1.3	59.9	O K
5760	min	Summer	9.348	0.348	1.3	42.1	O K
7200	min	Summer	9.258	0.258	1.3	31.3	O K
8640	min	Summer	9.200	0.200	1.2	24.2	O K
10080	min	Summer	9.161	0.161	1.2	19.5	O K

Storm Event		Rain (mm/hr)		Discharge Volume (m³)	Time-Peak (mins)	
15	min	Summer	156.101	0.0	54.1	16
30	min	Summer	103.795	0.0	71.6	31
60	min	Summer	66.103	0.0	91.8	62
120	min	Summer	40.250	0.0	111.8	122
180	min	Summer	29.726	0.0	123.9	182
240	min	Summer	23.809	0.0	132.3	242
360	min	Summer	17.216	0.0	143.5	360
480	min	Summer	13.656	0.0	151.8	480
600	min	Summer	11.385	0.0	158.1	600
720	min	Summer	9.798	0.0	163.1	656
960	min	Summer	7.701	0.0	170.7	770
1440	min	Summer	5.446	0.0	179.9	1024
2160	min	Summer	3.819	0.0	191.0	1448
2880	min	Summer	2.961	0.0	197.5	1848
4320	min	Summer	2.058	0.0	205.9	2680
5760	min	Summer	1.598	0.0	213.1	3344
7200	min	Summer	1.330	0.0	221.8	4032
8640	min	Summer	1.155	0.0	231.1	4672
10080	min	Summer	1.034	0.0	241.4	5352

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File	Checked by AW	Diamage
XP Solutions	Source Control 2020.1.3	

	Stor Even		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status
15	min	Winter	9 441	0 441	1.3	53.4	ОК
		Winter			1.3		ОК
60	min	Winter	9.729		1.3		Flood Risk
120	min	Winter	9.863		1.3		Flood Risk
180	min	Winter	9.929	0.929	1.4		Flood Risk
240	min	Winter	9.966	0.966	1.4	116.9	Flood Risk
360	min	Winter	9.993	0.993	1.4	120.2	Flood Risk
480	min	Winter	9.997	0.997	1.4	120.6	Flood Risk
600	min	Winter	9.987	0.987	1.4	119.5	Flood Risk
720	min	Winter	9.970	0.970	1.4	117.3	Flood Risk
960	min	Winter	9.922	0.922	1.4	111.6	Flood Risk
1440	min	Winter	9.836	0.836	1.3	101.2	Flood Risk
2160	min	Winter	9.708	0.708	1.3	85.7	Flood Risk
2880	min	Winter	9.589	0.589	1.3	71.3	ОК
4320	min	Winter	9.340	0.340	1.3	41.2	O K
5760	min	Winter	9.194	0.194	1.2	23.5	O K
7200	min	Winter	9.123	0.123	1.2	14.8	O K
8640	min	Winter	9.086	0.086	1.1	10.4	O K
10080	min	Winter	9.071	0.071	1.0	8.6	ОК

Storm Event		Rain (mm/hr)		Discharge Volume (m³)	Time-Peak (mins)	
15	min	Winter	156.101	0.0	54.1	16
30	min	Winter	103.795	0.0	71.6	31
60	min	Winter	66.103	0.0	91.8	62
120	min	Winter	40.250	0.0	111.8	120
180	min	Winter	29.726	0.0	123.9	178
240	min	Winter	23.809	0.0	132.3	236
360	min	Winter	17.216	0.0	143.5	352
480	min	Winter	13.656	0.0	151.8	464
600	min	Winter	11.385	0.0	158.1	576
720	min	Winter	9.798	0.0	163.1	680
960	min	Winter	7.701	0.0	170.7	788
1440	min	Winter	5.446	0.0	179.9	1082
2160	min	Winter	3.819	0.0	191.0	1540
2880	min	Winter	2.961	0.0	197.5	1992
4320	min	Winter	2.058	0.0	206.0	2724
5760	min	Winter	1.598	0.0	213.1	3344
7200	min	Winter	1.330	0.0	221.8	3968
8640	min	Winter	1.155	0.0	231.1	4584
10080	min	Winter	1.034	0.0	241.4	5192

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## Rainfall Details

Rainfall Model						FEH
Return Period (years)						100
FEH Rainfall Version						2013
Site Location	GB	301481	381595	SJ	01481	81595
Data Type						Point
Summer Storms						Yes
Winter Storms						Yes
Cv (Summer)						1.000
Cv (Winter)						1.000
Shortest Storm (mins)						15
Longest Storm (mins)						10080
Climate Change %						+40

#### Time Area Diagram

Total Area (ha) 0.139

Time (mins) Area From: To: (ha)

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#### Model Details

Storage is Online Cover Level (m) 10.000

#### Tank or Pond Structure

Invert Level (m) 9.000

## Depth (m) Area (m<sup>2</sup>) Depth (m) Area (m<sup>2</sup>)

0.000 121.0 1.000 121.0

#### Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0056-1400-1000-1400 Design Head (m) 1.000 Design Flow (1/s) 1.4 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Yes Diameter (mm) 56 Invert Level (m) 8.995 Minimum Outlet Pipe Diameter (mm) 75 Suggested Manhole Diameter (mm) 1200

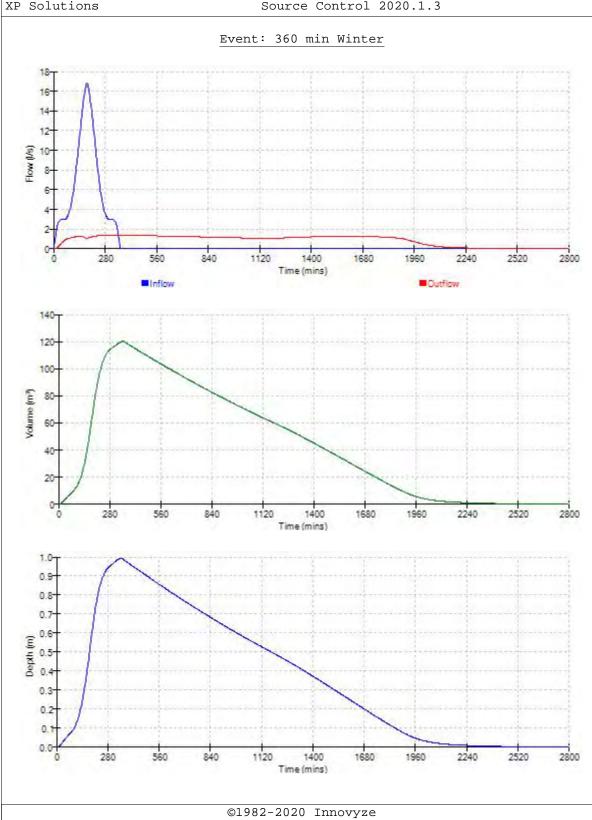
#### Control Points Head (m) Flow (1/s)

Design	Point	(Calculated)	1.000	1.4
		Flush-Flo™	0.245	1.3
		Kick-Flo®	0.497	1.0
Mean F	low ove	er Head Range	_	1.2

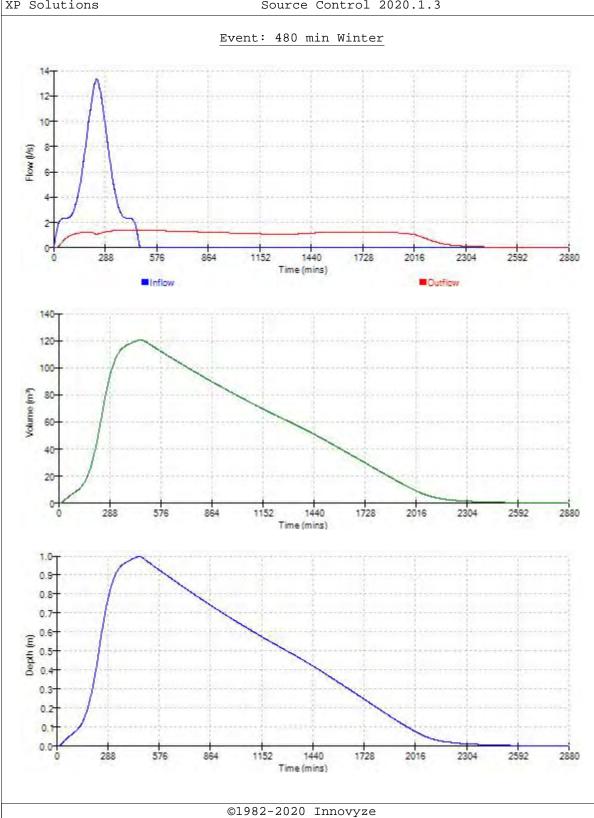
The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (1/s)	Depth (m)	Flow (1/s)	Depth (m)	Flow (1/s)
0.100	1.1	1.600	1.7	5.000	2.9
0.200	1.2	1.800	1.8	5.500	3.1
0.300	1.2	2.000	1.9	6.000	3.2
0.400	1.2	2.200	2.0	6.500	3.3
0.500	1.0	2.400	2.1	7.000	3.4
0.600	1.1	2.600	2.2	7.500	3.5
0.800	1.3	3.000	2.3	8.000	3.6
1.000	1.4	3.500	2.5	8.500	3.7
1.200	1.5	4.000	2.6	9.000	3.8
1.400	1.6	4.500	2.8	9.500	3.9

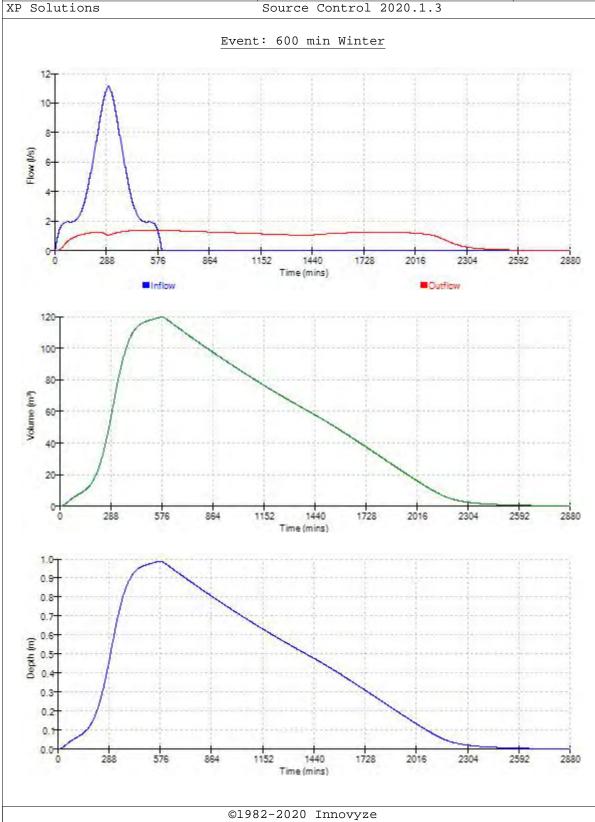
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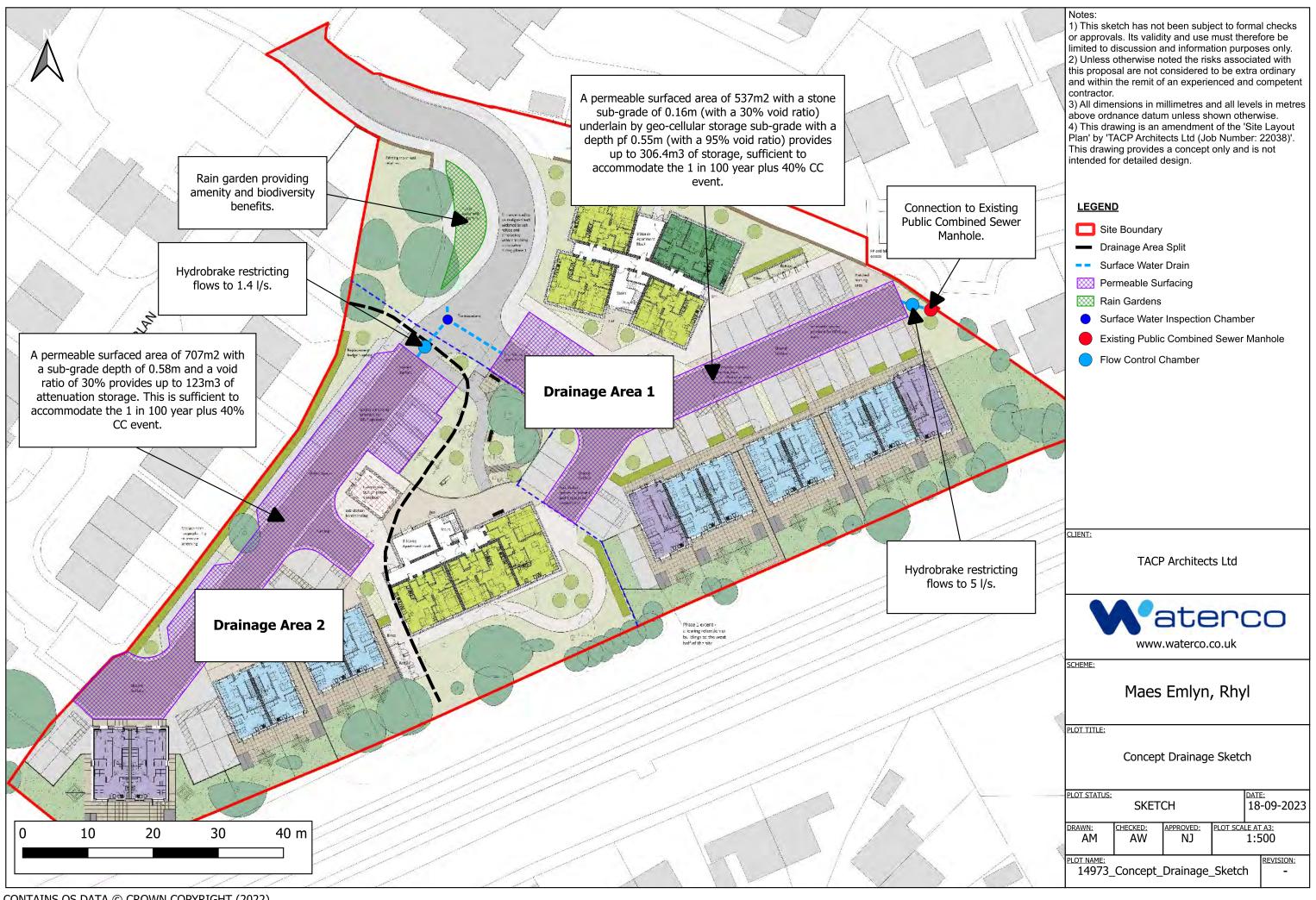


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# Appendix N Concept Drainage Sketch





# Appendix O Maintenance Schedules





# **Operation and Maintenance Requirements for Permeable Paving**

Maintenance Schedule	Required Action	Typical Frequency	
Regular maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment	
Occasional	Stabilise and move contributing and adjacent areas	As required	
maintenance	Removal of weeds or management using glyphospate applied directly into the weeds by an applicator rather than spraying	As required – once per year on less frequently used pavements	
Remedial actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50mm of the level or the paving	As required	
actions	Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)	
	Inspect for evidence of poor operation and / or weed growth – if required, take remedial action	Three-monthly, 48hr after large storms in first six months	
Monitoring	Inspect silt accumulation rates and establish appropriate brushing frequencies	Annually	
	Monitor inspection chambers	Annually	

Ref. Table 20.15, CIRIA C753 'The SuDS Manual'

The maintenance requirements detailed above are to be undertaken by the site owner.

Name	:	
Position	:	
Date	:	
Signed on behalf of the site owner	:	
	-	



# **Operation and Maintenance Requirements for Attenuation Storage Tanks**

Maintenance Schedule	Required Action	Typical Frequency
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary	Annually
	Remove sediment from pre-treatment structures and/ or internal forebays	Annually, or as required
Remedial actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build-up and remove If necessary	Every 5 years or as required

Ref. Table 21.3, CIRIA C753 'The SuDS Manual'

The maintenance requirements detailed above are to be undertaken by the site owner.			
Name	:		
Position	:		
Date	:		
Signed on behalf of the site owner	:		

# Appendix P Concept Designers Risk Assessment (cDRA)







Project:	Maes Emlyn, Rhyl	Maes Emlyn, Rhyl			14973
Client:	TACP Architects Ltd			_	
Report Reference:	14973				
Prepared by:	Adam McCulloch	Date:	28/02/2023		
Checked by:	Aled Williams	Date:	28/02/2023		
Reviewed by:	Nigel Jones	Date:	28/02/2023		

#### Requirement:

The Construction (Design and Management) Regulations 2015 (CDM 2015) place an obligation on the Designer to take all reasonable steps to provide, with the design, sufficient information about the design, construction or maintenance of the structure, to adequately assist the client, other designers and contractors to comply with their duties under CDM. The Designer has undertaken this assessment to identify any extra-ordinary risks, or those that would not be expected on this particular project by an experienced and competent Contractor. The aim is to avoid needless paperwork and bureaucracy and ensure the assessment is project specific, relevant and proportionate to the risk.

#### **DRA Summary**

Each of the following risk areas has been considered using the question below. Is a risk present which is considered to be extra-ordinary or unexpected in this instance?

If YES - A detailed risk assessment is required at design stage

If UNKNOWN - Insufficient information has been provided at concept design stage and the risks are unknown. Further consideration must be given at design stage(s)

If **NO** - No further action is required.

Hazard Ref.	Risk Areas	YES, UNKNOWN or NO	Comments
1	Ground Conditions	Unknown	
2	Hazardous Environment	Unknown	
3	Existing Working Environment	Unknown	
4	Existing Services	Yes	Existing services in place - See GPR Survey
5	Proximity to Other Structure(s)	Yes	Railway line to south
6	Near Waterbody / flood risk	No	Low flood risk, see FCA reference: 14973-FCA-01
7	Proximity to Other Activities	Unknown	Railway line to south
8	Sequence of Construction	Unknown	
9	Access	Unknown	Provided off Churton Road
10	Interfaces	Unknown	
11	Confined Space Working	Unknown	
12	Maintenance Considerations	Unknown	
13	Working at Height	Unknown	
14	Steep Slopes	No	The site is generally flat with levels ranging from 7.08m AOD to 6.45m AOD
15	Demolition / Refurbishment / Repair	Yes	Demolition of existing residential properties
16	Welfare	Unknown	
17	Occupational Health	Unknown	
18	Environmental Issues	Unknown	
19	Other Significant Hazards not Identified Above	Unknown	
20	Residual Risk to Future Users	Unknown	